

Tulloch Engineering Inc.

New Water Intake and Low Lift Pumping Station, Blind River, Ontario

HYDROGEOLOGICAL INVESTIGATION



CIMA+ file number: T001592B
23 April 2025



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Table of contents

1. Introduction.....	3
1.1 Site Description.....	3
1.2 Objective.....	3
2. Methodology	4
2.1 Borehole Advancement and Monitoring Well Installation	5
2.2 Test Well Development.....	5
2.3 Pumping Test	5
2.4 Single Well Response Testing	6
2.5 Groundwater Level Monitoring.....	6
2.6 Groundwater Sampling.....	6
3. Background	7
3.1 Land Use and Servicing.....	7
3.2 Aerial Photography & Satellite Imagery.....	7
3.3 Topography	7
3.4 Geology.....	8
3.4.1 Surficial Geology.....	8
3.4.2 Bedrock Geology.....	8
3.4.3 Hydrogeology	8
3.5 Surface Water Features and Areas of Natural and Scientific Interest	9
3.6 Source Water Protection.....	9
4. Field Investigation.....	9
4.1 Groundwater Level Monitoring.....	9
4.2 Pumping Test	10
4.3 Single Well Response Testing	10
4.4 Groundwater Sampling.....	11
5. Construction Dewatering Assessment	11
5.1 Excavation Parameters	11
5.2 Groundwater Infiltration.....	12
5.3 Permitting	14
6. Impact Assessment and Proposed Mitigations.....	14
6.1 Impact Assessment.....	14
6.2 Mitigation Measures	15
6.2.1 Management of Discharge	15
6.2.2 Mitigation Measures	16

6.2.3 Proposed Monitoring	16
7. Limiting Conditions.....	16
8. References	17

List of Tables

Table A - Monitoring Well Information
Table B - Pumping Test Results
Table C - Single Well Response Test Results
Table D - Dewatering Assessment Parameters
Table E - Infiltration Rates into Excavation

List of Appendices

Appendix A Figures
Appendix B Borehole Logs
Appendix C Single Well Response Test Analyses
Appendix D Tables
Appendix E Laboratory Certificate of Analysis
Appendix F Dewatering Calculations

1. Introduction

CIMA Canada Inc. (CIMA+) was retained by Tulloch Engineering Inc. (Tulloch) on behalf of the Town of Blind River (the 'Town') to complete a Hydrogeological Investigation to support the New Water Intake and Low Lift Pumping Station (LLPS) (the 'Project') associated with the Blind River Water Treatment Plant (WTP) located in Blind River, Ontario, herein referred to as (the 'Project').

1.1 Site Description

The Project Site is approximately 0.55 hectares, located adjacent to the existing Blind River WTP, bounded by Martin Street to the north, Lake Huron to the south and residential housing to the east. The centre of the Project Site is in UTM Zone 17T, with approximate coordinates of Easting 349268 m and Northing 5116291 m.

The Study Area for the hydrogeological investigation has been established herein based on industry standard practices to include the surrounding area within a 500-meter (m) radius of the Site boundaries to support baseline characterization of Site conditions and identification of potential receptors of any expected impacts where applicable.

The Project Site and Study Area are delineated as shown on **Figure 1 (Attachment A)** and described below.

1.2 Objective

The hydrogeological investigation aims to characterize the existing hydrogeological conditions at the Project Site to inform detailed design of the Low Lift Pumping Station (LLPS) for the Project. The purpose of the hydrogeological investigation is to:

- Characterize the subsurface soil and shallow bedrock groundwater conditions at the Site and within the Study Area, consisting of a 500 m radius from the planned excavation area.
- Assess the need for groundwater control and temporary construction dewatering and evaluate permitting requirements - Ministry of the Environment, Conservation and Parks (MECP) Permit to Take Water (PTTW) Application(s) or a water taking registration on the Environmental Activity and Sector Registry (EASR).
- Identify potential receptors, evaluate potential groundwater impacts attributable to construction dewatering, and provide recommended mitigation measures for construction dewatering.

2. Methodology

The following activities were completed as part of the hydrogeological investigation provided herein for the Project:

- Desktop review of public information sources, including but not limited to online water well database maintained by the Ministry of the Environment, Conservation and Parks (MECP), geological mapping prepared by the Ontario Geological Survey (OGS), watershed impact studies, Source Water Protection Atlas, and other publicly available information;
- Coordination with Walker Drilling Ltd. (Walker Drilling) and Tulloch for the installation of a 5-inch (127 mm) open hole test well completed to a depth of approximately 10 m below ground surface (bgs) and sealed into bedrock, and two 2-inch (50.8 mm) monitoring wells, including well development and recovery observations to inform pumping test design;
- Completion of a step-rate pumping test using an electric submersible pump on the newly constructed test well to determine physical hydrogeological properties of the shallow bedrock unit across the test well interval;
- Water level monitoring within the newly constructed test well and observation wells throughout the duration of the pumping test using pressure transducers and data loggers;
- Completion of single well response tests (SWRT) at the newly constructed overburden monitoring well for evaluation of the horizontal hydraulic conductivity of the subsurface material at the Project Site;
- Collection of groundwater samples for comparison to the applicable criteria;
- Completion of a dewatering assessment;
- Development of groundwater mitigation, monitoring, and contingency plan(s), if required; and,
- Recommendations for additional studies and or assessments to support permitting and/or construction activities.

2.1 Borehole Advancement and Monitoring Well Installation

A drilling program was completed at the Project Site between October 22 and 23, 2024 by Walker Drilling under the direction of Tulloch and CIMA+. A total of three (3) boreholes were completed by Tulloch. BH-100 located adjacent to the future building footprint was advanced as a 5-inch (127 mm) borehole into bedrock to a total depth of 10.68 m bgs using PW/PQ drilling equipment. The borehole was sealed into bedrock encountered at a depth of 4.57 m bgs and completed as an open hole test well. BH-101 and BH-102 were advanced as 2-inch (50.8 mm) boreholes using HW/HQ drilling equipment. BH-101 was advanced to bedrock at 11.48 m bgs and installed as a 2-inch PVC monitoring well with a 10-foot (3.05 m) screen extending from 8.43 to 11.48 m bgs. BH-102 was advanced to 7.16 m bgs and installed as a 2-inch PVC monitoring well with a 5-foot (1.52 m) screen extending from 5.03 to 6.55 m bgs. Borehole logs including well installation details are included in **Appendix B**.

2.2 Test Well Development

Under the direction of Tulloch and CIMA+, on November 1, 2024 Walker Drilling completed well development and collection of initial recovery observations at the newly constructed BH-100 test well to determine the appropriate pumping rate for the planned pumping test. The test well was pumped manually with 5/8-inch Waterra tubing for approximately 30 minutes until the well was dry. Recovery was monitored via manual water level measurements using a Solinst® Model 101 electric water level tape for approximately 60 minutes following completion of development.

2.3 Pumping Test

On November 21, 2024, CIMA+ conducted a step-rate pumping test on the test well BH-100. The test was conducted with a stainless-steel Monsoon® submersible pump installed at a depth of 11.21 m below the top of casing. Water level information was collected using Solinst® Levelogger transducers and data loggers, as supplemented by manual water level measurements using a Solinst® Model 101 electric water level tape. A barometric transducer (Solinst® Barologger Model 3001) was utilized to permit water level data correction to changes in barometric pressure.

The test well was pumped at 3.79 L/min (1 USGPM) initially and was increased to 7.57 L/min (2 USGPM) after seven (7) minutes and was maintained for the remainder of the pumping test, which was terminated after 30 minutes when the water level had reached the pump intake. Water level recovery data was recorded until the water level reached approximately 95% of its original elevation, which was achieved after 180 minutes following pump shutoff. Results of the pumping test was analysed using the AQTESOLV Pro software and the Theis (1935) conceptual model and the Cooper-Jacob (1945) approximation.

An additional verification pumping test was performed on monitoring well BH-101 on November 22, 2024 following the methodology described above, yielding comparable results to those observed at BH-100.

2.4 Single Well Response Testing

Single well response testing (SWRT) was completed by CIMA+ personnel at the newly installed overburden monitoring well (BH-102) on November 21, 2024 to determine the horizontal hydraulic conductivity of the overburden material at the Project Site. The in-situ hydraulic testing was completed following standard slug and bail test methodology, involving the rapid raising (slug test) or lowering (bail test) of the hydraulic head using an object of known volume followed by the recording of the recovery response until the water level well has returned to within at least 90% of its original elevation.

Water level information was collected using Solinst® Levelogger transducers and data loggers, as supplemented by manual water level measurements using a Solinst® Model 101 electric water level tape. A barometric transducer (Solinst® Barologger Model 3001) was utilized to permit water level data correction to changes in barometric pressure. Results of the hydraulic testing were analysed using the AQTESOLV Pro software and the Hvorslev (1951) method.

2.5 Groundwater Level Monitoring

CIMA+ personnel conducted groundwater level monitoring at all newly constructed well locations at the Project Site on November 21, 2024. All manual water level measurements collected by CIMA+ personnel were collected using a Solinst® Model 101 electric water level tape relative to the top of casing reference elevation.

2.6 Groundwater Sampling

Groundwater samples were collected from the bedrock test well (BH-100) and overburden monitoring well BH-102 by CIMA+ personnel on November 21, 2024, for analysis of select general chemistry parameters for preliminary assessment of potential dewatering constraints in comparison to the Provincial Water Quality Objectives (PWQO) published by the Ministry of Environment and Energy (MOEE. 1994).

Samples were collected using standard groundwater sampling methodology in conjunction with parameter stabilization. Groundwater samples were collected from BH-100 using a stainless-steel Monsoon® submersible pump powered by a 12V portable battery in combination with standard Waterra® 5/8" x 1/2" LDPE tubing, while the sample collected from BH-102 was obtained the standard Waterra® LDPE tubing and inertial lift foot-valve. Groundwater quality field parameters were measured using a Horiba® U-52 multi-parameter instrument. Groundwater samples were collected once parameter stabilization was achieved.

Groundwater samples were submitted under strict chain-of-custody protocols to AGAT Laboratories Ltd. (AGAT) in Mississauga, Ontario, a Canadian Association for Laboratory Accreditation Inc. (CALA) accredited laboratory.

3. Background

3.1 Land Use and Servicing

The Project Site is located on Martin Street and along on the shores of Lake Huron within the Town of Blind River, south of Highway 17 and east of Blind River as indicated in **Figure 1 (Appendix A)**. Existing land use within the surrounding Study Area is dominated by industrial/commercial properties to the north and west, as well as residential properties to the east.

Based on the information presented in the Town of Blind River Official Plan (2015), the Project Site is located within an area zoned as Open Space. It is noted that the Official Plan designates areas of shoreline along Lake Huron and Blind River as being Environmental Protection (EP) zones; select EP zones fall within 200 m of the Project Site.

Based on available mapping information and a reconnaissance of the Project Site and surrounding area, it's assumed that the residential areas adjoining the Project Site are fully serviced with municipal water supply and sewer services.

3.2 Aerial Photography & Satellite Imagery

Current and historical aerial photographs obtained from the Land Information Ontario (LIO) interactive map tool and Google Earth Pro (accessed January, 2025) were used to conduct a preliminary desktop review of land usage in the immediate vicinity of the Project Site for identification of potential sources of environmental impacts to groundwater.

In addition to the existing WTP adjacent to the Project Site and commercial properties to the north, information gathered from the desktop review identified a property located at 75 Causley Street, at the corner of Huron Avenue and Causley Street/Hwy 17, which appears to have contained a building which was demolished between 2009 and 2019. The nature of the former building is unknown however a series of groundwater monitoring wells appear to have been installed surrounding the property, suggesting potential environmental concern. The property is approximately 150 m northeast of the Project Site.

3.3 Topography

Review of the regional topographic and drainage mapping information presented by the Ministry of Natural Resources and Forestry (MNRF) (accessed January, 2025) indicates that topography across the Project Site slopes gradually to the south towards Lake Huron. Ground surface elevations within the Project Site range from a maximum of approximately 185 metres

(m) above sea level (asl) on the north side of the Site, decreasing to approximately 180 m asl along the shores of Lake Huron to the south. No notable topographic features are indicated on or in proximity to the Project Site based on the available mapping information. The regional topography is shown in **Figure 2 (Appendix A)**.

3.4 Geology

3.4.1 Surficial Geology

Surficial geology mapping presented by the OGS (2010) and reproduced within **Figure 3 (Appendix A)** indicates that the surficial geology over the Project Site is dominated by Pleistocene till, undifferentiated, predominantly sand to silty sand matrix, high content of clasts, often low in matrix carbonate content. Records from the Water Well Information System (WWIS) indicate that overburden material locally is generally described as sand and gravel fill material overlaying silt and clay.

Ontario Soil Surveys Report no. 50, Soils of Blind River – Sault Ste Marie Area includes some mapping of the Study Area. Available soil mapping classifies soil as orthic humo-ferric podzol, described as noncalcareous very stony sand and/or clay lacustrine. The soil type is classified as 'well' drained.

3.4.2 Bedrock Geology

Regional bedrock geology mapping information presented by the OGS (2007) indicates that the bedrock underlying the overburden material within the Project Site has been classified as the McKim Formation in the Elliot Lake Group, characterized as siltstone, wacke and argillite. OGS bedrock geology mapping is reproduced within **Figure 4 (Appendix A)**.

Observations from the November 2024 drilling program completed at the Site indicated that bedrock was encountered at depths ranging from 4.57 to 6.43 m bgs.

3.4.3 Hydrogeology

Based on the available information and watershed mapping, regional groundwater flow within the overburden and bedrock material is inferred to follow regional ground surface topography sloping to the south towards Lake Huron.

A search of the WWIS indicated a total thirteen (13) water well records for locations within a 250 meter (m) radius of the Project Site boundaries, as presented in **Figure 5 (Appendix A)**. Review of the information indicates that all records correspond with monitoring wells and test holes completed in the overburden, to depths up to approximately 4 m bgs. No static water level information or yield assessment was available.

Additional details pertaining to hydrogeological observations and results from the field investigations are included in **Section 4**.

3.5 Surface Water Features and Areas of Natural and Scientific Interest

The Project Site is not located within the jurisdiction of a Conservation Authority, however as noted in **Section 3.1**, the Town of Blind River Official Plan designates areas of shoreline along Lake Huron and Blind River as being Environmental Protection (EP) zones; select EP zones fall within 200 m of the Project Site.

Review of the Ontario Watershed Information Tool (OFAT, accessed March, 2025) presented by the MNR indicates that surficial drainage in the vicinity of the Project Site is predominantly oriented towards the south, following gently sloping topography towards Lake Huron.

3.6 Source Water Protection

Based on the online interactive mapping information included within the Source Protection Information Atlas (MECP, accessed March 2025), no Source Water Protection measures have been developed within proximity to the Project Site.

4. Field Investigation

4.1 Groundwater Level Monitoring

CIMA+ personnel conducted groundwater level monitoring at all newly constructed well locations at the Project Site during the November 21, 2024 hydrogeological investigation event. Water level depths were measured to range from 2.83 m bgs at the open hole bedrock test well (BH-100) to 3.61 m bgs at the shallow overburden monitoring well (BH-102). Calculated water level elevations were determined to be relatively consistent between monitoring intervals, ranging from 176.75 to 176.97 m asl. Monitoring well construction details and groundwater elevations collected by CIMA+ are included below in **Table A**. Borehole locations are presented on **Figure 6 (Appendix A)**.

Table A: Monitoring Well Information

Monitoring Location	Ground Surface Elevation	Top of Bedrock		Top of Screen		Bottom of Screen		Water Level November 21, 2024	
		m asl	m asl	m bgs	m asl	m bgs	m asl	m bgs	m asl
BH-100 ¹	179.58	175.01	4.57	175.01 ²	4.57	168.90 ³	10.68	176.75	2.83
BH-101	180.38	173.95	6.43	171.95	8.43	168.90	11.48	176.97	3.41
BH-102	180.38	173.95	6.43	178.86	5.03	173.83	6.55	176.77	3.61

bgs - below ground surface

asl - above sea level

1 - open hole test well

2 - bottom of casing

3 - end of borehole

4.2 Pumping Test

On November 21, 2024 CIMA+ conducted a step-rate pumping test on the open hole bedrock test well BH-100. The test well was pumped at 3.79 L/min (1 USGPM) initially and was increased to 7.57 L/min (2 USGPM) after seven (7) minutes and was maintained for the remainder of the pumping test, which was terminated after 30 minutes when the water level had reached the pump intake. Water level recovery data was recorded until the water level reached approximately 95% of its original elevation, which was achieved after 180 minutes following pump shutoff.

Results of the pumping test were analysed using the AQTESOLV Pro software and the Theis (1935) conceptual model and the Cooper-Jacob (1945) approximation for an unconfined aquifer condition. Results from the pumping test are summarized below in **Table B**. The individual pumping test analyses are included in **Appendix C**.

An additional verification pumping test was performed on monitoring well BH-101 on November 22, 2024, yielding comparable results to those observed at BH-100.

Table B: Pumping Test Results

Monitoring Location	Analysis ¹	Calculated Transmissivity (m ² /s)	Mean Transmissivity (m ² /s)
BH-100	Theis	1.32 E-06	2.67 E-06
	Cooper-Jacob	4.02 E-06	

Notes:

- 1 Theis (1935) Pumping Test Solution
Cooper-Jacob (1945) Pumping Test Solution

4.3 Single Well Response Testing

Single well response testing (SWRT) was completed by CIMA+ personnel at the newly installed overburden monitoring well (BH-102) on November 21, 2024 to determine the horizontal hydraulic conductivity of the subsurface material at the Project Site. Results of the hydraulic testing were analysed using the AQTESOLV Pro software and the Hvorslev (1951) method. Results of the single well response tests are summarized below in **Table C**. The individual SWRT analyses are included in **Appendix C**.

The hydraulic testing at BH-102 yielded a mean hydraulic conductivity result of 1.54E-05 m/s, which is typical of the inferred sand to silty sand till overburden material in which the monitoring well was screened.

Table C: Single Well Response Test Results

Monitoring Location	Screened Interval (m bgs)	Screened Geological Unit	Test ID	Hydraulic Conductivity (K) ¹ (m/s)	Mean Hydraulic Conductivity (K) (m/s)
BH-102	5.03 to 6.55	Till - sand to silty sand	Slug 1	1.82E-05	1.54E-05
			Bail 1	1.35E-05	
			Slug 2	1.52E-05	
			Bail 2	1.48E-05	

Notes:

1 - Hvorslev (1951) aquifer test solution

4.4 Groundwater Sampling

Groundwater samples were collected from the test well (BH-100) and monitoring well BH-102 by CIMA+ personnel on November 21, 2024, for analysis of select general chemistry parameters for preliminary assessment of potential dewatering constraints in comparison to the Provincial Water Quality Objectives (PWQO) published by the Ministry of Environment and Energy (MOEE. 1994). The groundwater quality results are presented in **Table 1 (Appendix D)** in comparison to the applicable criteria. Laboratory Certificates of Analysis are included as **Appendix E**.

Total cobalt and total copper were reported in exceedance of the PWQO for samples collected at both BH-100 and BH-102, while total vanadium, total phosphorous, and chromium VI at BH102, and total silver at BH-100, were also reported in exceedance of the PWQO.

5. Construction Dewatering Assessment

5.1 Excavation Parameters

It is understood that open cut construction methods will be used for excavation of the LLPS. The details of the trench excavation summarized below in **Table D** have been interpreted based on the available information in the 90% Detailed Design plans completed by CIMA+ (November 2024). Based on the available information, the base of the excavation required to support the planned work is assumed to be approximately 7.2 m bgs. To maintain dry working conditions within the excavations it is assumed water levels will have to be lowered to an elevation 1 m below the base of all excavations during the planned work.

Table D: Dewatering Assessment Parameters

Excavation Specifications	Depth m bgs	Elevation m asl
Ground Surface	-	180.56
Groundwater Elevation ¹	1.0	179.56
Bedrock Surface	4.57	175.01
Bottom of excavation - Wet Well ²	7.17	173.39
Maximum Groundwater Elevation under Dewatering	8.17	172.39
Required Drawdown (m)	7.17	

Notes: 1 Conservative high water level estimate
2 Tulloch (November 2024)

5.2 Groundwater Infiltration

In consideration of groundwater infiltration contributions from the overburden unit, the mean hydraulic conductivity value of 1.54E-05 m/s described in **Section 4.3** above was evaluated as being representative of the inferred sand to silty sand till unit for use in the construction dewatering assessment herein.

With respect to groundwater infiltration contributions from the bedrock unit, hydraulic conductivity was estimated using the mean Transmissivity (T) m²/s value of 2.67E-06 m²/s calculated as described in **Section 4.2** based on the analyses of the pumping test conducted at BH-100 in conjunction with an assumed aquifer thickness of 10 m. Hydraulic conductivity is related to Transmissivity as follows:

$$K = T/b$$

Where:

K = Hydraulic conductivity of the geological unit (m/s)

T = Transmissivity (m²/s)

b = Aquifer Thickness (m), set as 10 m

Based on the above calculation, hydraulic conductivity within the bedrock unit is estimated to be 2.67E-07 m/s. The hydraulic conductivity value is considered to be reflective of a conservative assessment of groundwater infiltration, based on comparison to typical theoretical hydraulic conductivity values for siltstone and field observations during the hydrogeological investigation.

The infiltration rate calculations provided herein are based on the following assumptions:

- The geological material on-Site is isotropic and as a result the hydraulic conductivity of the stratigraphic unit is uniform.

- Vertical flow is negligible and there is no upward hydraulic pressure or confined/artesian conditions. The bedrock aquifer is inferred to be unconfined based on observations from the field program and aquifer testing.
- The infiltration of groundwater is isolated from other sources including nearby streams or bodies of water.
- No other dewatering or pumping activity in the vicinity of the site will impact groundwater inflow rates during the excavation.

Infiltration rate calculations were performed using the Dupuit (1863) and Forchheimer (1930) method (presented in Powers et. al., 2007) for estimating steady-state groundwater flow to a point source under unconfined aquifer conditions, as described below. The Dupuit-Forchheimer approximation assumes that for the excavation areas, the vertical flow is negligible, and that the groundwater discharge is proportional to the saturated aquifer thickness, driven by the slope of the water table.

$$Q = \frac{\pi K * (H^2 - h_w^2)}{\ln \frac{R_o + r_w}{r_w}}$$

Where:

Q = Groundwater inflow rate into the excavation (m³/s)

K = Hydraulic conductivity of the geological unit (m/s)

H = Saturated thickness prior to dewatering (m)

h_w = Saturated thickness after dewatering (m)

R_o = Radius of influence from point source (m)

r_w = Effective radius of point source (m)

The effective radius (r_w) is approximated by the equation below:

$$r_w = \sqrt{\frac{ab}{\pi}}$$

a = Length of the excavation, assumed to be 13.5 m

b = Width of the excavation (m), assumed to be 13 m

The radius of influence (R_o) is approximated by the method described by Sichardt et Kyrieleis (1930):

$$R_o = 3000(H - h_w)\sqrt{K}$$

Calculations and results from the dewatering assessment are included in **Appendix F**. A summary of the estimated infiltration rates is presented in **Table E** along with an applied safety factor of 3 to compensate for potential components of additional inflow not captured within the above equations, including incidental precipitation events.

Table E: Infiltration Rates into Excavation

Dewatering Assessment	Estimated infiltration rate (m ³ /day)	
LLPS Excavation	Estimated infiltration rate	42
	Applied Safety Factor (x3)	127

Based on the calculations presented in **Appendix F** and summarized in **Table D** above, the vertical and lateral groundwater inflow from the walls and base of the planned excavation, with dimensions of 13 x 13.5 m and a depth of 7.17 m bgs, located within the Project Site is estimated to be approximately 127 m³/day (23 USGPM) inclusive of the applied safety factor of 3 to compensate for potential components of additional inflow not captured within the above equations, including incidental precipitation events. The associated predicted radius of influence was estimated to be 54 m in the overburden unit, and 4 m in the bedrock unit.

5.3 Permitting

The construction dewatering assessment presented in **Section 5.2** above anticipates conservative construction dewatering needs of up to 172 m³/day, dependent on excavation dimensions and location within the Project Site.

Based on the assessment herein, a water taking registration on the Environmental Activity and Sector Registry (EASR) would be required to facilitate Project construction dewatering activities. A Permit to Take Water (PTTW) is not anticipated to be required.

6. Impact Assessment and Proposed Mitigations

6.1 Impact Assessment

An impact assessment was completed to evaluate the potential for adverse impact resulting from the planned Project construction dewatering activities. The Site setting, land use, as well as geological and hydrogeological Site conditions were taken into consideration. The following assessment is provided based on the planned Project activities and their potential impacts:

- The predicted radius of influence associated with planned dewatering activities was estimated to be 54 m in the overburden unit, and 4 m in the bedrock unit as presented in **Appendix F**.
- Based on available mapping information and a reconnaissance of the Project Site and surrounding area, it's assumed that the residential areas adjoining the Project Site are fully serviced with municipal water supply and sewer services and therefore no impacts to the municipal water supply or other water users are anticipated as a result of the planned Project activities.
- Based on the online interactive mapping information included within the Source Protection Information Atlas (MECP, accessed March 2025), no Source Water Protection measures have been developed within proximity to the Project Site.
- Desktop environmental review of properties within the Study Area identified a series of groundwater monitoring wells located at 75 Causley Street suggesting potential environmental concern at the property. Given the approximately 150 m distance between the property and the Project Site, and the relatively small radius of influence calculated as part of the dewatering assessment (54 m in overburden and 4 m in bedrock), the potential for environmental concern at 75 Causley Street is not anticipated to represent a constraint or impact planned Project construction dewatering activities.
- Potential impacts to the surrounding environment related to water taking and water discharge as a result of the required dewatering activities for the Project will be addressed through the development of a Water Taking and Discharge Plan as required by the EASR registration. It is noted that select total metals and general chemistry parameters (chromium VI, cobalt, copper vanadium, silver, and total phosphorous) were reported in exceedance of the PWQO for groundwater samples collected among newly installed test and monitoring wells and should be considered as part of the anticipated Water Taking and Discharge Plan in the event of anticipated discharge to the environment.
- Geotechnical assessment of potential settlement as a result of dewatering is not anticipated to be required based on the absence of existing structures within the estimated radius of influence.

6.2 Mitigation Measures

6.2.1 Management of Discharge

It is recommended that a Water Taking and Discharge Plan be developed in advance of construction dewatering activities, as required by the EASR registration for construction dewatering, including assessment of dewatering effluent discharge location(s) and treatment requirements in consideration of the identified PWQO exceedances for select total metals and general chemistry parameters including chromium VI, cobalt, copper vanadium, silver, and total phosphorous.

6.2.2 Mitigation Measures

During dewatering discharge, it is recommended that the Contractor develop and implement best management practices to mitigate any potential impacts to the environment. Mitigation measures may include but not be limited to:

- Erosion and Sediment Control (ESC) measures (silt fences etc.);
- Stormwater management (efforts to direct surface water flow away from the excavation etc.);
- Spill containment and response plan; and,
- Proper storage and management of excavated soils for reuse and/or off-Site disposal.

6.2.3 Proposed Monitoring

It is recommended that a monitoring program (quality and quantity) be developed in advance of the construction dewatering activities in consideration of the anticipated EASR registration and associated Water Taking and Discharge Plan. The monitoring program should consider Project specific dewatering practices, the discharge location, and available intact monitoring wells.

7. Limiting Conditions

CIMA+ completed diligent and reasonable research in the conduct of this evaluation, with respect to the recognized laws and standards of practice.

The facts presented in this report are strictly limited to the period of investigation. The conclusions presented in this report are based on the available information and documents, the observations made during the Site visit and the information obtained from communications with various contacts. The interpretation presented in this report is limited to this data.

CIMA+ is not responsible for erroneous conclusions due to voluntary abstention or the non-availability of pertinent information. Any opinion expressed in relation to legal or regulatory conformity is technical and should not be, in any case, considered as legal advice.

CIMA+ has prepared this report for the sole use of the client. Any use of this report by a third party, as any decision based on this report, is the singular responsibility of the third party. CIMA+ will not be held responsible for eventual damages towards a third party resulting from decisions taken, or based, on this report.

8. References

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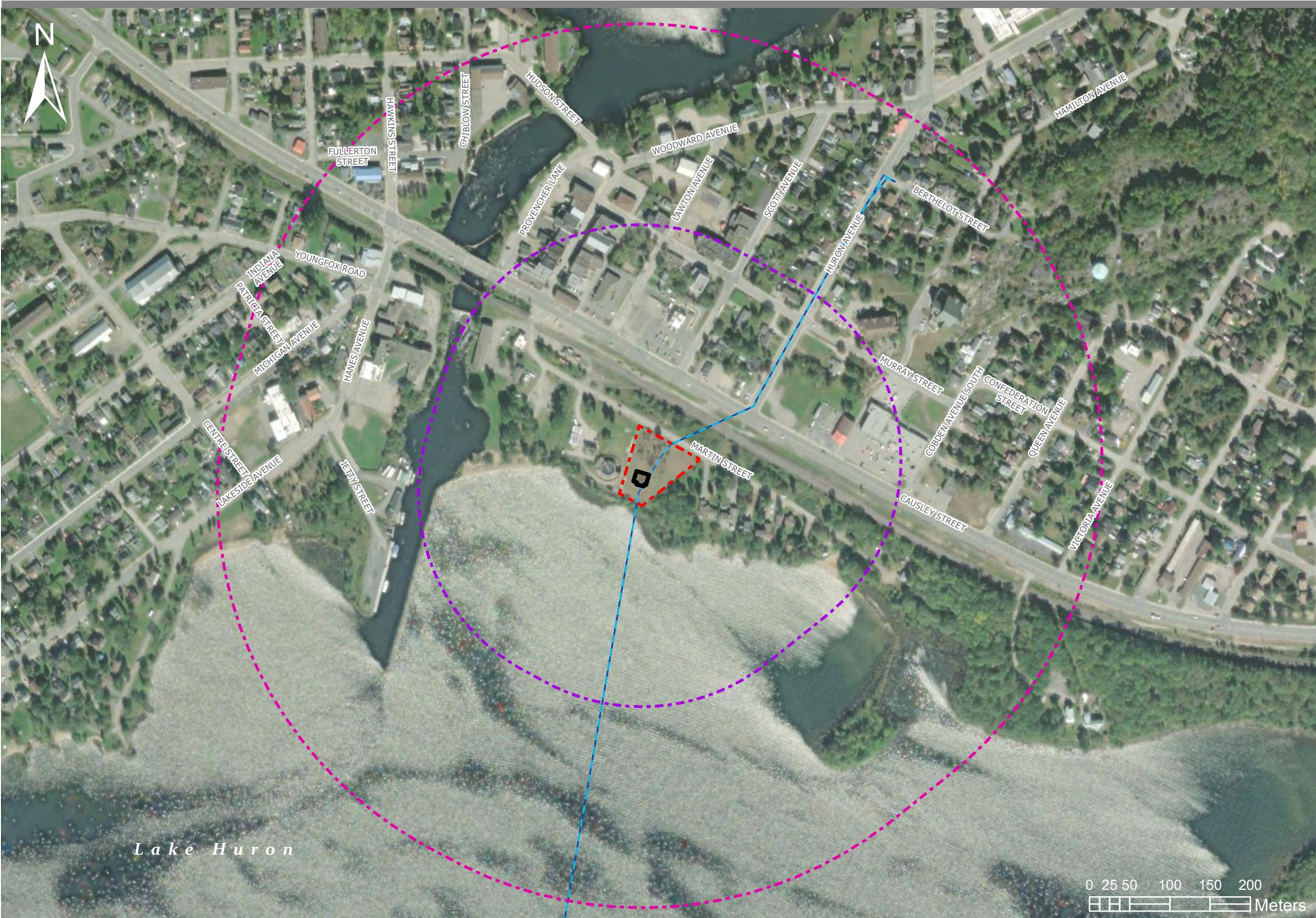
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A

Appendix A Figures



■ Project Site
■ Study Area - 250 m
■ Study Area - 500 m
■ Proposed Building Footprint
— Proposed Water Intake



Spatial Reference:
 Name: NAD 1983 CSRS MTM 13
 GCS: GCS North American 1983 CSRS
 Datum: North American 1983 CSRS
 Map Units: Meter
 Scale: 1:5,000

Sources:
 - Road Labels, ORN, 2024
 - Topographic Contours, Watercourse, LIO, 2024
 - Basemap : Source: Esri, Maxar, Earthstar Geographics, and the GIS User Community, Province of Ontario, Ontario MNR, Esri Canada, Esri, HERE, Garmin, INCREMENT P, USGS, METI/NASA, EPA,

General Notes:
 Dimensions on the plan should be read and not measured.
 Any errors or omissions should be reported to CIMA+. The boundaries, areas, and title deeds must be verified by a surveyor.

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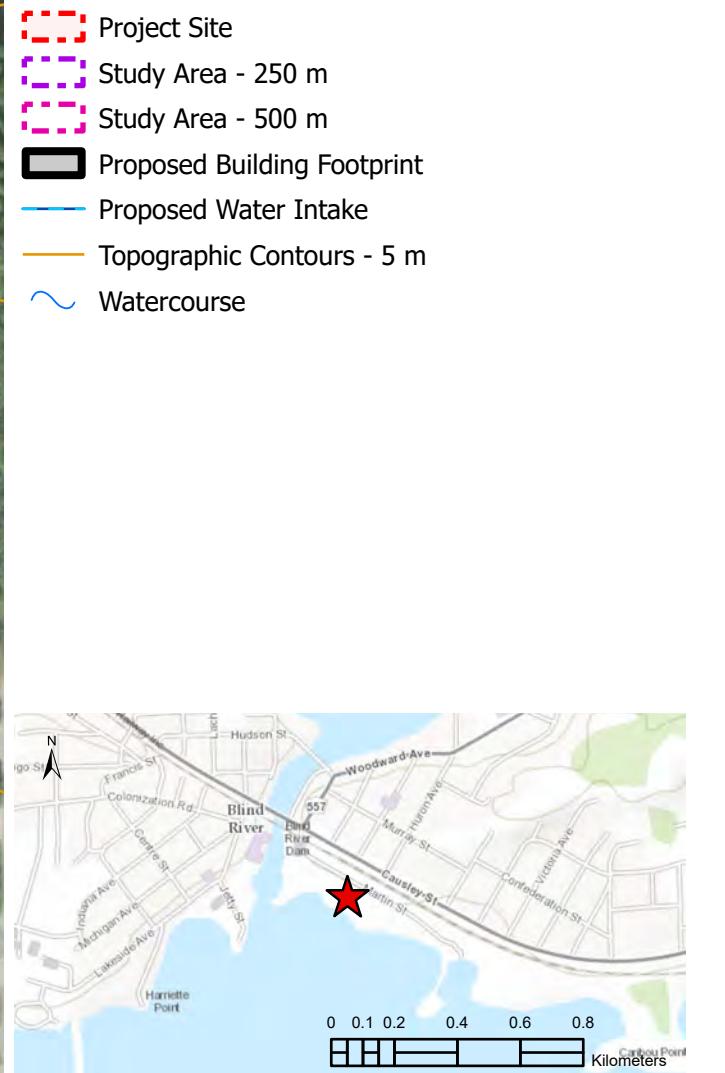
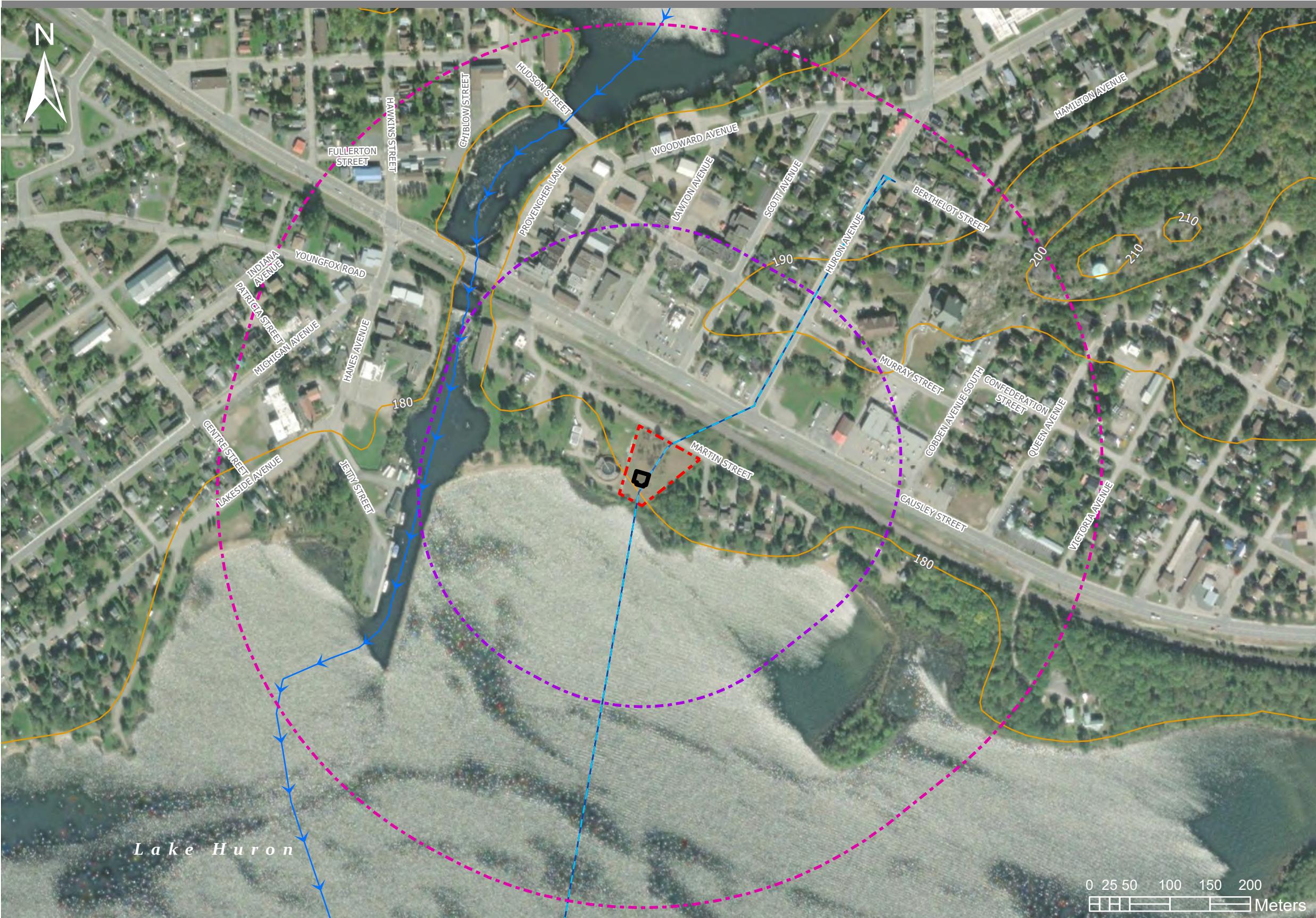
Figure 1 - Site Location Map

Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
 17 T 4850065 N; 5116292 E, Blind River, Ontario
 Tulloch Engineering Inc.

Ref # : T001592B

Survey by : -
 Figure by : S. Scott
 Concept by : S. Scott
 Verified by : M. Klein

CIMA+



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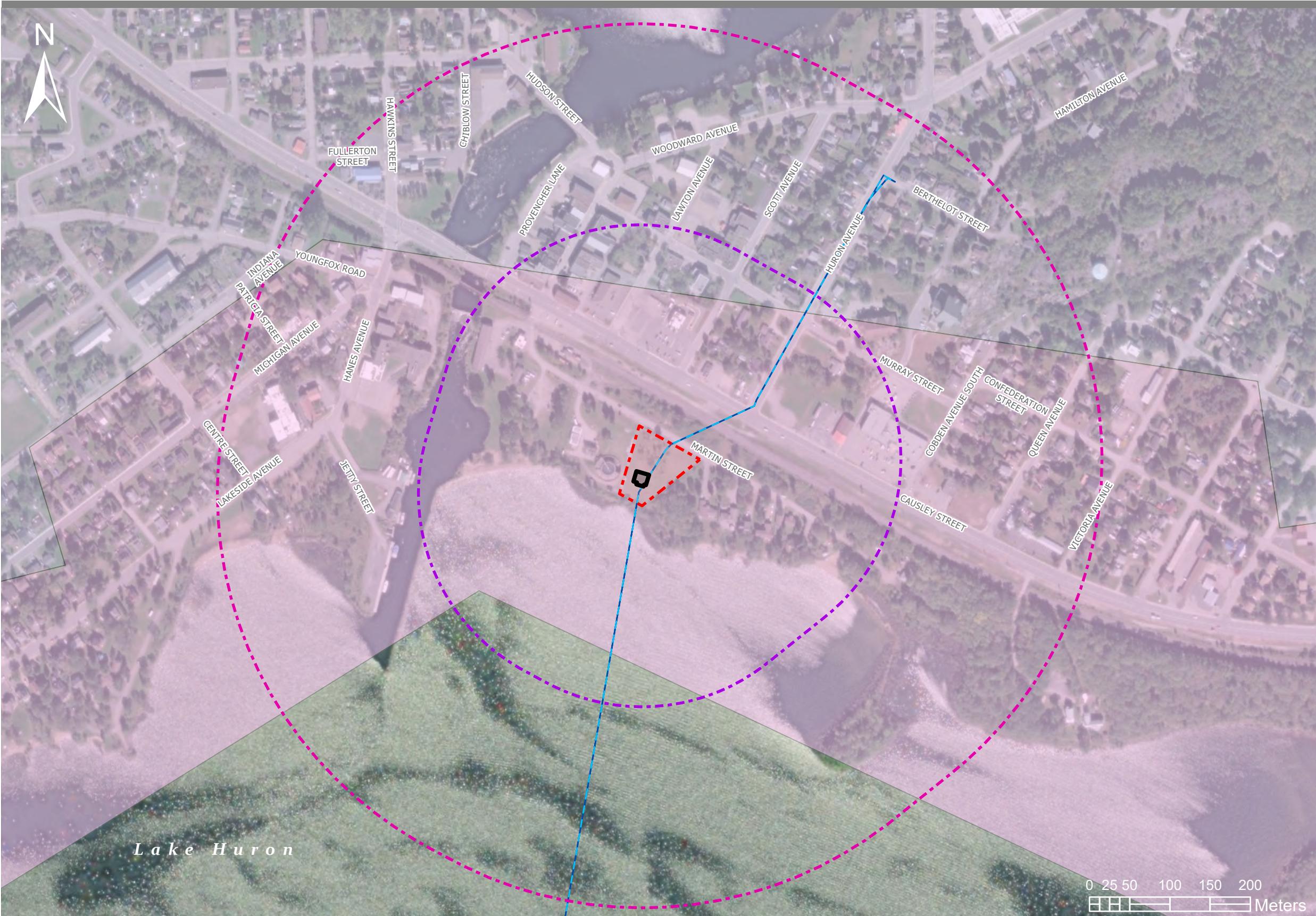
Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
 17 T 4850065 N; 5116292 E, Blind River, Ontario
 Tulloch Engineering Inc.

Survey by : -
 Figure by : S. Scott
 Concept by : S. Scott
 Verified by : M. Klein

Ref # : T001592B

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Revision 00 - Issued for report - April 23, 2025 10:37:58 AM



Project Site
Study Area - 250 m
Study Area - 500 m
Proposed Building Footprint
Proposed Water Intake

Quaternary Geology

Bedrock
Till



Sources:

- Road Labels, ORN, 2024
- Ontario Geological Survey 2000. Quaternary geology of Ontario; Ontario Geological Survey, ERLIS Data Set —EDS014-REV.
- Basemap : Province of Ontario, Ontario MNR, Esri Canada, Esri, HERE, Garmin, INCREMENT P, USGS,

General Notes:

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Figure 3 - Surficial Geology Map

Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
17 T 4850065 N; 5116292 E, Blind River, Ontario
Tulloch Engineering Inc.

Ref # : T001592B

Survey by : -
Figure by : S. Scott
Concept by : S. Scott
Verified by : M. Klein

CIMA+

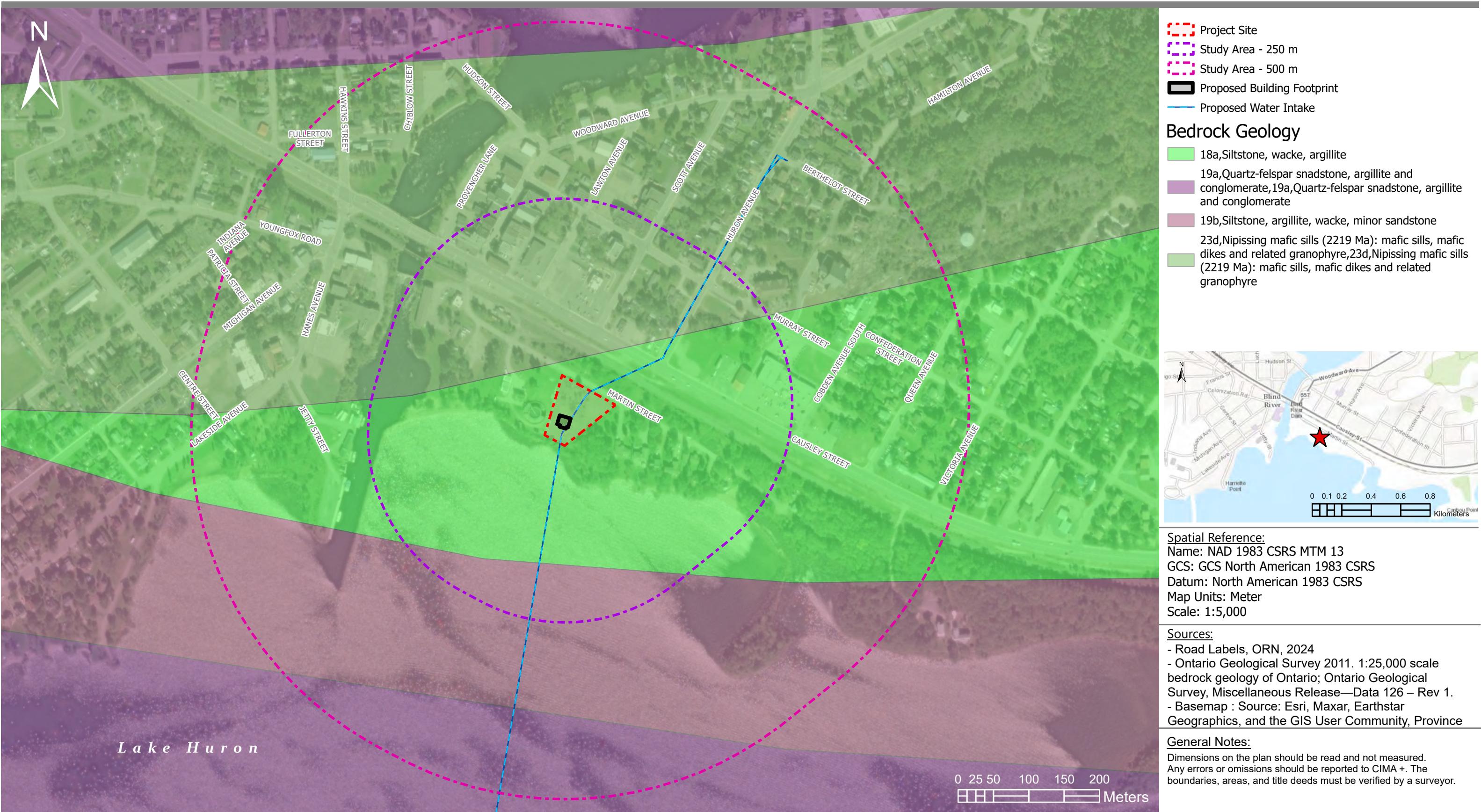


Figure 4 - Bedrock Geology Map

Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
 17 T 4850065 N; 5116292 E, Blind River, Ontario
 Tulloch Engineering Inc.

Ref # : T001592B

Survey by : -
 Figure by : S. Scott
 Concept by : S. Scott
 Verified by : M. Klein

CIMA+

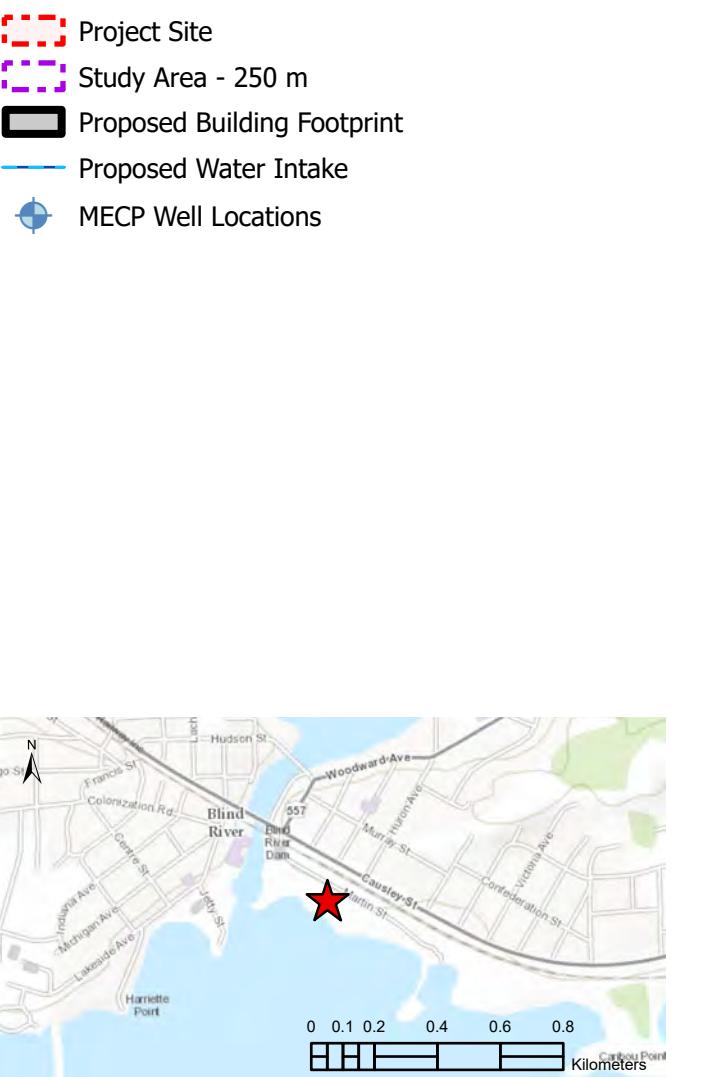


Figure 5 - MECP Well Locations

This plan is the intellectual property of "CIMA+"; any total or partial reproduction is subject to the explicit prior agreement of an employee of "CIMA+".

Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
 17 T 4850065 N; 5116292 E, Blind River, Ontario
 Tulloch Engineering Inc.

Survey by : -
 Figure by : S. Scott
 Concept by : S. Scott
 Verified by : M. Klein

Ref # : T001592B

CIMA+

Revision 00 - Issued for report - April 23, 2025 10:41:32 AM



Figure 6 - Monitoring Well Locations

Hydrogeological Investigation - New Water Intake & Huron Ave. Reconstruction
17 T 4850065 N; 5116292 E, Blind River, Ontario
Tulloch Engineering Inc.

Ref # : T001592B

- Project Site
- Proposed Building Footprint
- Proposed Water Intake
- Monitoring Well Locations (CIMA+, 2024)



Spatial Reference:
Name: NAD 1983 CSRS MTM 13
GCS: GCS North American 1983 CSRS
Datum: North American 1983 CSRS
Map Units: Meter
Scale: 1:750

Sources:
- Road Labels, ORN, 2024
- Basemap : Maxar, Microsoft, Province of Ontario, Ontario MNR, Esri Canada, Esri, HERE, Garmin, INCREMENT P, USGS, METI/NASA, NGA, EPA, USDA, AAFC, NRCan

General Notes:
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Survey by : -
Figure by : S. Scott
Concept by : S. Scott
Verified by : M. Klein

CIMA+

B

Appendix B Borehole Logs

Consultant: CIMAT	Client: Town of Blind River	Page 1 of 1																																																																																																																																		
BOREHOLE LOGGING																																																																																																																																				
Borehole N° BH-100																																																																																																																																				
<p>Project name : Blind River Intake Location Study</p> <p>Applicant :</p> <p>Location : Blind River, Ontario, Canada</p> <p>Drilling contractor : Walker Drilling Ltd.</p> <p>Type of drill : CME</p> <p>Borehole diameter : PW</p> <p>Dip : </p> <p>Strike : </p> <p>Executed by : Tulloch Engineering Inc.</p> <p>Core diameter : PQ</p> <p>Compiled by : J. Tardioli</p> <p>Verified by : M. Klein</p>																																																																																																																																				
<p>Coordinates (m) X: 349246.0 17 () Y: 5116289.0 Z: 179.58</p> <p>Project No. : T001592B</p> <p>Localization figure No. :</p> <p>Drilling start date : 2024-10-22</p> <p>Total borehole depth : 10.68 m</p>																																																																																																																																				
<table border="1"> <thead> <tr> <th>SAMPLE TYPE</th> <th>TERMINOLOGY</th> <th>ROC QUALITY DESIGNATION</th> <th>COMPACTION</th> <th>INDEX "N"</th> <th>GROUNDWATER</th> </tr> </thead> <tbody> <tr> <td>SS Split Spoon</td> <td>"Traces" 1-10%</td> <td>% RQD <25</td> <td>Very loose</td> <td>0-4</td> <td></td> </tr> <tr> <td>DC Diamond rock core</td> <td>"Some" 10-20%</td> <td>25-50</td> <td>Loose</td> <td>4-10</td> <td></td> </tr> <tr> <td>MA Manual sampling</td> <td>adjective (...y) 20-35%</td> <td>50-75</td> <td>Poor</td> <td>10-30</td> <td></td> </tr> <tr> <td>AS Auger</td> <td>"and" 35-50%</td> <td>75-90</td> <td>Fair</td> <td>30-50</td> <td></td> </tr> <tr> <td>ST Shelby tube</td> <td></td> <td>90-100</td> <td>Good</td> <td>>50</td> <td></td> </tr> <tr> <td>TW Thin wall sampler</td> <td></td> <td></td> <td>Excellent</td> <td></td> <td></td> </tr> <tr> <td colspan="2">SAMPLE STATE</td> <td>SYMBOLS</td> <td>SOIL CLASSIFICATION</td> <td>CONSISTENCY</td> <td>SHEAR STRENGTH (Cu)</td> </tr> <tr> <td colspan="2">  Remoulded  Intact (thin wall sampler)  Lost  Core (diamond rock core) </td> <td> N: Standard penetration value R: Refusal (N > 100) Woh: Weight of Hammer / 61 cm R.Q.D: Rock quality designation $\% \text{ R.Q.D} = \frac{\sum \text{cores} > 4 \text{ po. (10 cm)}}{\text{Drilled length}}$ </td> <td> Clay < 0,002 mm Silt 0,002 to 0,080 mm Sand 0,080 to 5 mm Gravel 5 to 80 mm Cobbles 80 to 300 mm Boulders > 300mm </td> <td> Very soft Soft Firm Stiff Very stiff Hard </td> <td> <12 kPa 12-25 kPa 25-50 kPa 50-100 kPa 100-200 kPa >200 kPa </td> </tr> </tbody> </table>			SAMPLE TYPE	TERMINOLOGY	ROC QUALITY DESIGNATION	COMPACTION	INDEX "N"	GROUNDWATER	SS Split Spoon	"Traces" 1-10%	% RQD <25	Very loose	0-4		DC Diamond rock core	"Some" 10-20%	25-50	Loose	4-10		MA Manual sampling	adjective (...y) 20-35%	50-75	Poor	10-30		AS Auger	"and" 35-50%	75-90	Fair	30-50		ST Shelby tube		90-100	Good	>50		TW Thin wall sampler			Excellent			SAMPLE STATE		SYMBOLS	SOIL CLASSIFICATION	CONSISTENCY	SHEAR STRENGTH (Cu)	 Remoulded  Intact (thin wall sampler)  Lost  Core (diamond rock core)		N: Standard penetration value R: Refusal (N > 100) Woh: Weight of Hammer / 61 cm R.Q.D: Rock quality designation $\% \text{ R.Q.D} = \frac{\sum \text{cores} > 4 \text{ po. (10 cm)}}{\text{Drilled length}}$	Clay < 0,002 mm Silt 0,002 to 0,080 mm Sand 0,080 to 5 mm Gravel 5 to 80 mm Cobbles 80 to 300 mm Boulders > 300mm	Very soft Soft Firm Stiff Very stiff Hard	<12 kPa 12-25 kPa 25-50 kPa 50-100 kPa 100-200 kPa >200 kPa																																																																												
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<p>Remark(s):</p> <p>Cima_Log_en_official_CLEAN.sty</p>																																																																																																																																				
Creation date 2025-04-22																																																																																																																																				

BOREHOLE LOGGING

Borehole N°

BH-101Project name : **Blind River Intake Location Study**

Coordinates (m)

X: **349282.0**

17()

Y: **5116294.0**Z: **180.38**

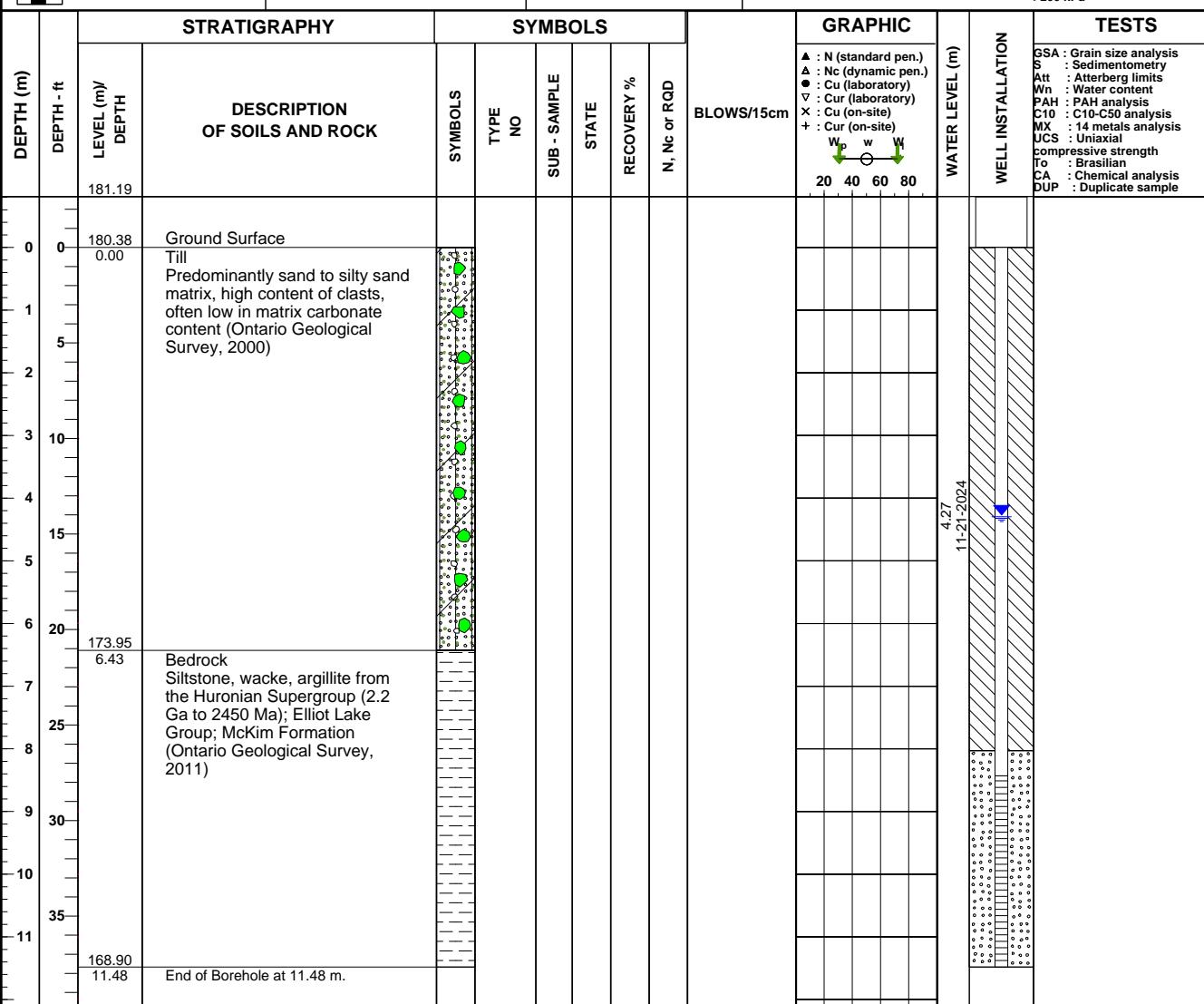
Applicant :

Location : **Blind River, Ontario, Canada**Drilling contractor : **Walker Drilling Ltd.**Type of drill : **CME**Borehole diameter : **HW**Dip : **Strike :**Executed by : **Tulloch Engineering Inc.**Core diameter : **HQ**Compiled by : **J. Tardioli**Verified by : **M. Klein**Project No. : **T001592B**

Localization figure No. :

Drilling start date : **2024-10-22**Total borehole depth : **11.48 m**

SAMPLE TYPE	TERMINOLOGY	ROC QUALITY DESIGNATION	COMPACTION	INDEX "N"	GROUNDWATER
SS Split Spoon		% RQD 1-10%	<25 Very loose	0-4	
DC Diamond rock core	"Traces"	10-20%	25-50 Poor	4-10	
MA Manual sampling	"Some"	20-35%	50-75 Fair	10-30	
AS Auger	adjective (...y)	35-50%	75-90 Good	30-50	
ST Shelby tube	"and"		90-100 Excellent	Very dense >50	
TW Thin wall sampler					Date: 2024-11-21 Date:
					Depth (m): 4.27 Depth (m):
SAMPLE STATE	SYMBOLS	SOIL CLASSIFICATION	CONSISTENCY	SHEAR STRENGTH (Cu)	
Remoulded	N: Standard penetration value	Clay < 0,002 mm	Very soft	<12 kPa	
Intact (thin wall sampler)	R: Refusal (N > 100)	Silt 0,002 to 0,080 mm	Soft	12-25 kPa	
Lost	Woh: Weight of Hammer / 61 cm	Sand 0,080 to 5 mm	Firm	25-50 kPa	
Core (diamond rock core)	R.Q.D: Rock quality designation	Gravel 5 to 80 mm	Stiff	50-100 kPa	
	% R.Q.D = Σ Cores > 4 po. (10 cm) Drilled length	Cobbles 80 to 300 mm	Very stiff	100-200 kPa	
		Boulders > 300mm	Hard	>200 kPa	



BOREHOLE LOGGING

Borehole N°

BH-102Project name : **Blind River Intake Location Study**

Coordinates (m)

X: **349283.0**

17()

Y: **5116295.0**Z: **180.38**

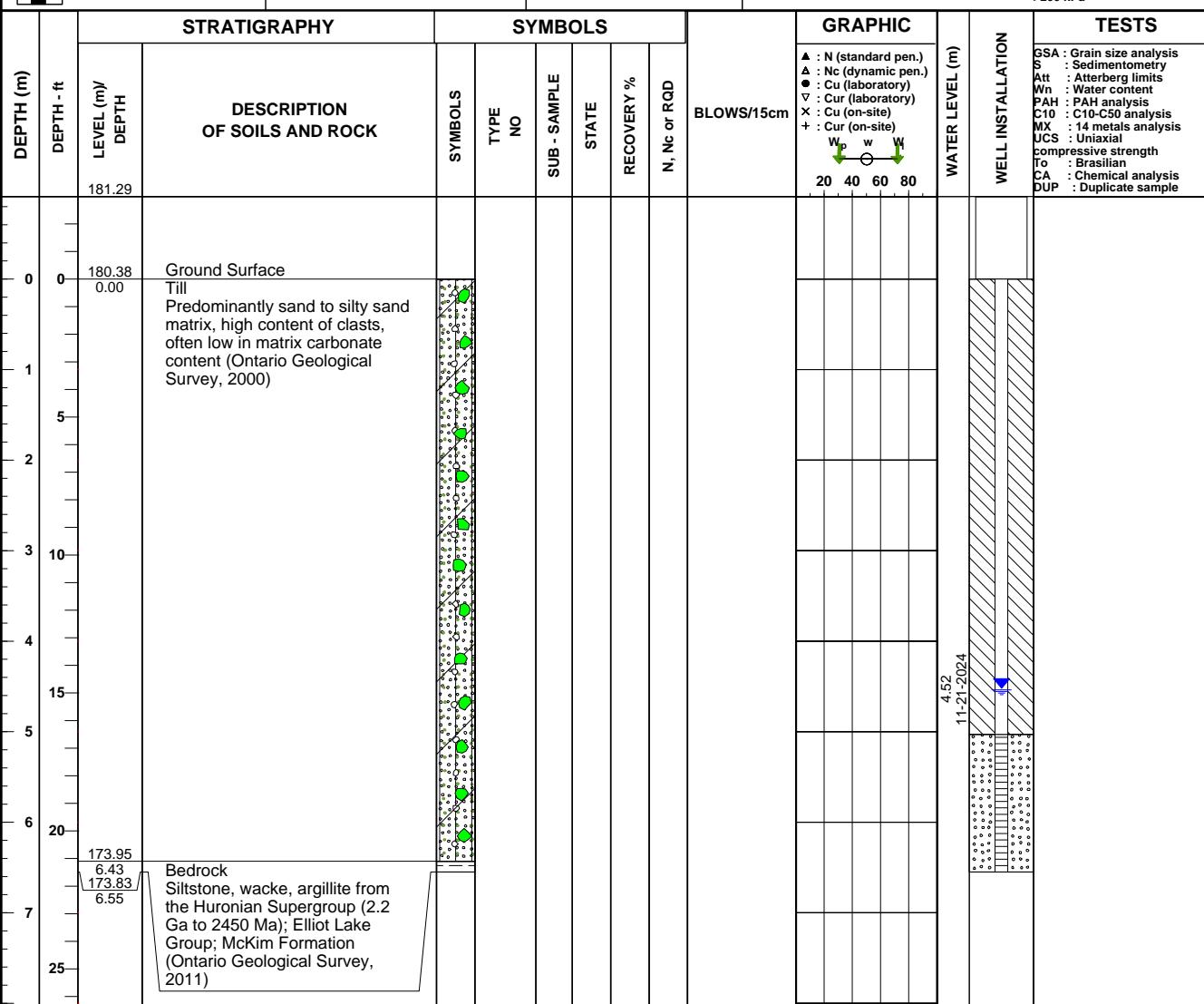
Applicant :

Location : **Blind River, Ontario, Canada**Drilling contractor : **Walker Drilling Ltd.**Type of drill : **Unknown**Borehole diameter : **HW**Dip : **Strike :**Executed by : **Tulloch Engineering Inc.**Core diameter : **HQ**Compiled by : **J. Tardioli**Verified by : **M. Klein**Project No. : **T001592B**

Localization figure No. :

Drilling start date : **2024-10-23**Total borehole depth : **6.55 m**

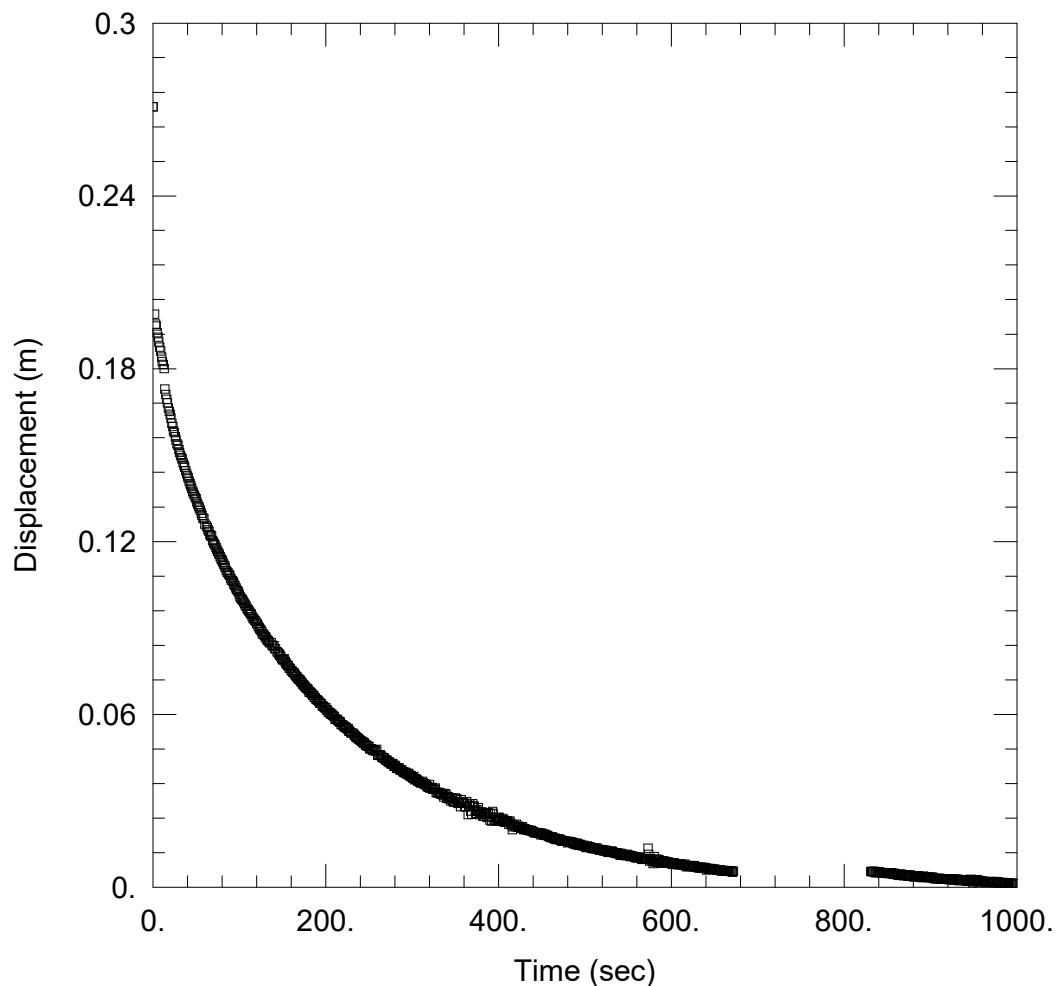
SAMPLE TYPE	TERMINOLOGY	ROC QUALITY DESIGNATION	COMPACTION	INDEX "N"	GROUNDWATER
SS Split Spoon	"Traces" 1-10%	<25 Very poor	Very loose	0-4	
DC Diamond rock core	"Some" 10-20%	25-50 Poor	Loose	4-10	
MA Manual sampling	adjective (...y) 20-35%	50-75 Fair	Compact	10-30	
AS Auger	"and" 35-50%	75-90 Good	Dense	30-50	
ST Shelby tube		90-100 Excellent	Very dense	>50	
TW Thin wall sampler					
SAMPLE STATE	SYMBOLS	SOIL CLASSIFICATION	CONSISTENCY	SHEAR STRENGTH (Cu)	
 Remoulded	N: Standard penetration value	Clay < 0,002 mm	Very soft	<12 kPa	
 Intact (thin wall sampler)	R: Refusal (N > 100)	Silt 0,002 to 0,080 mm	Soft	12-25 kPa	
 Lost	Woh: Weight of Hammer / 61 cm	Sand 0,080 to 5 mm	Firm	25-50 kPa	
 Core (diamond rock core)	R.Q.D: Rock quality designation	Gravel 5 to 80 mm	Stiff	50-100 kPa	
	% R.Q.D = Σ Cores > 4 po. (10 cm) / Drilled length	Cobbles 80 to 300 mm	Very stiff	100-200 kPa	
		Boulders > 300mm	Hard	>200 kPa	



Remark(s):

C

Appendix C Single Well Response Test Analyses



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Slug 1 Linear vs draw.aqt
Date: 03/31/25

Time: 10:58:25

PROJECT INFORMATION

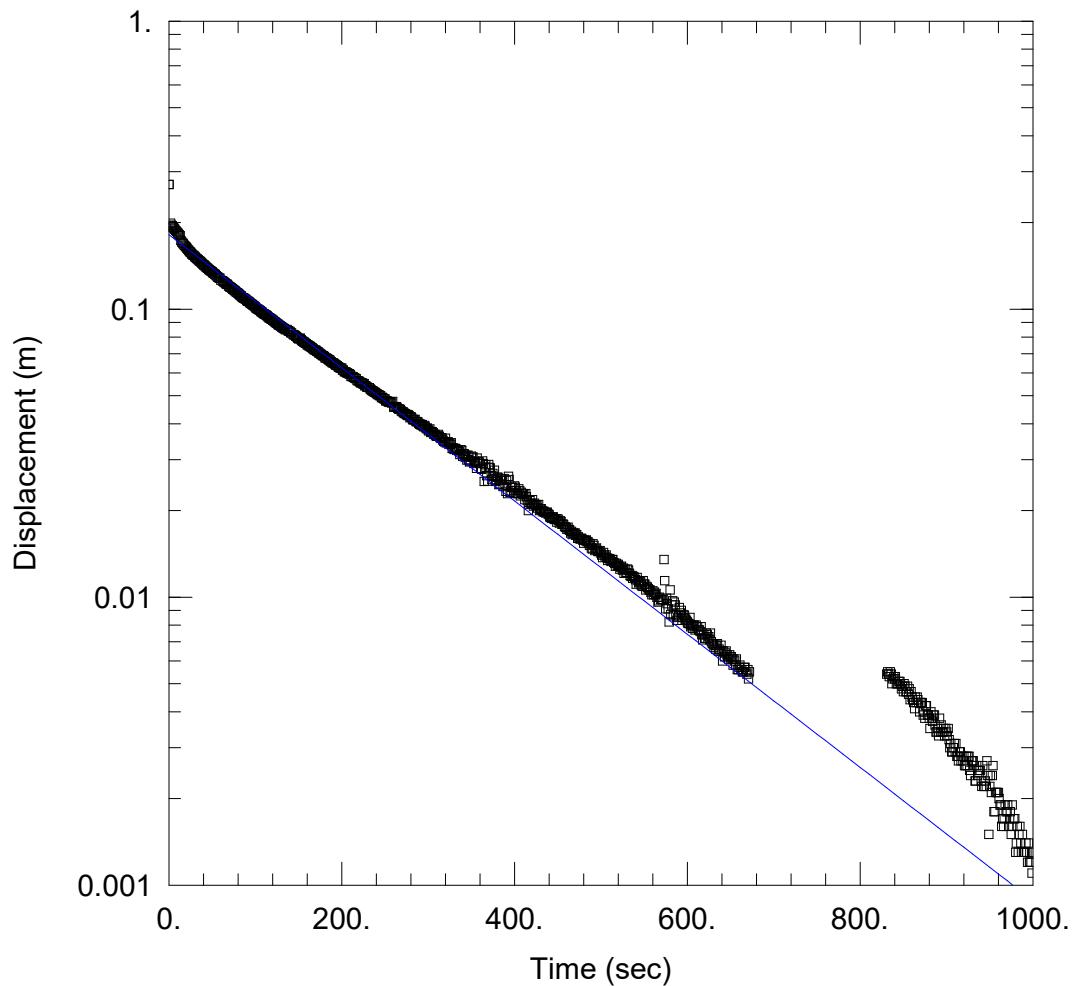
Company: CIMA+
Project: T001592B
Location: Blind River, ON
Test Well: BH-102
Test Date: 2024-11-21

AQUIFER DATA

Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: 0.271 m Static Water Column Height: 3.57 m
Total Well Penetration Depth: 2.034 m Screen Length: 1.524 m
Casing Radius: 0.0505 m Well Radius: 0.0254 m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Slug 1 Hvorslev unconfined.aqt
 Date: 03/31/25 Time: 11:00:49

PROJECT INFORMATION

Company: CIMA+
 Project: T001592B
 Location: Blind River, ON
 Test Well: BH-102
 Test Date: 2024-11-21

AQUIFER DATA

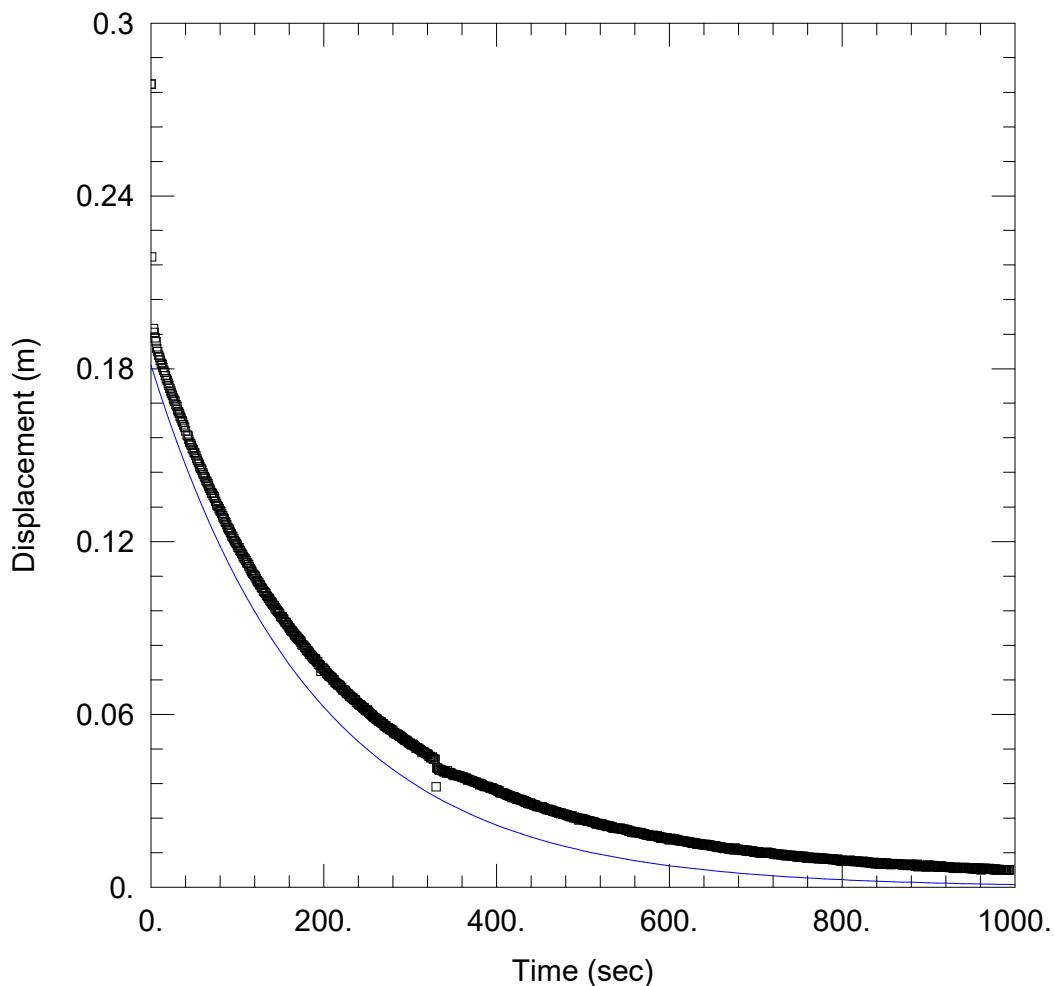
Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: <u>0.271</u> m	Static Water Column Height: <u>3.57</u> m
Total Well Penetration Depth: <u>2.034</u> m	Screen Length: <u>1.524</u> m
Casing Radius: <u>0.0505</u> m	Well Radius: <u>0.0254</u> m

SOLUTION

Aquifer Model: <u>Unconfined</u>	Solution Method: <u>Hvorslev</u>
<u>K</u> = <u>1.824E-5</u> m/sec	<u>y0</u> = <u>0.1812</u> m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Bail 1 Linear vs draw.aqt
 Date: 03/31/25

Time: 11:03:30

PROJECT INFORMATION

Company: CIMA+
 Project: T001592B
 Location: Blind River, ON
 Test Well: BH-102
 Test Date: 2024-11-21

AQUIFER DATA

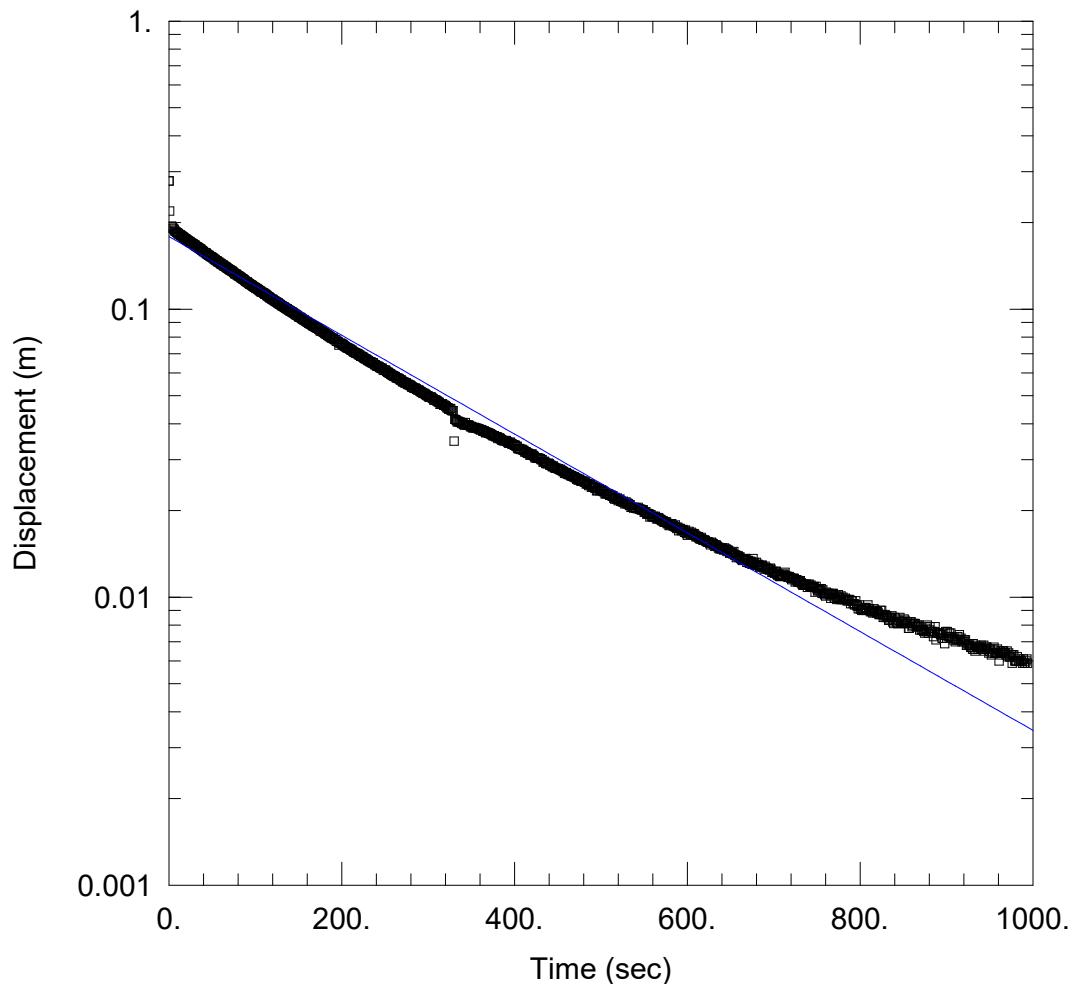
Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: 0.2788 m Static Water Column Height: 3.57 m
 Total Well Penetration Depth: 2.034 m Screen Length: 1.524 m
 Casing Radius: 0.0505 m Well Radius: 0.0254 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev
 $K = 1.824E-5$ m/sec $y_0 = 0.1812$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Bail 1 Hvorslev unconfined.aqt

Date: 03/31/25

Time: 11:09:09

PROJECT INFORMATION

Company: CIMA+

Project: T001592B

Location: Blind River, ON

Test Well: BH-102

Test Date: 2024-11-21

AQUIFER DATA

Saturated Thickness: 3.57 m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: 0.2788 m

Static Water Column Height: 3.57 m

Total Well Penetration Depth: 2.034 m

Screen Length: 1.524 m

Casing Radius: 0.0505 m

Well Radius: 0.0254 m

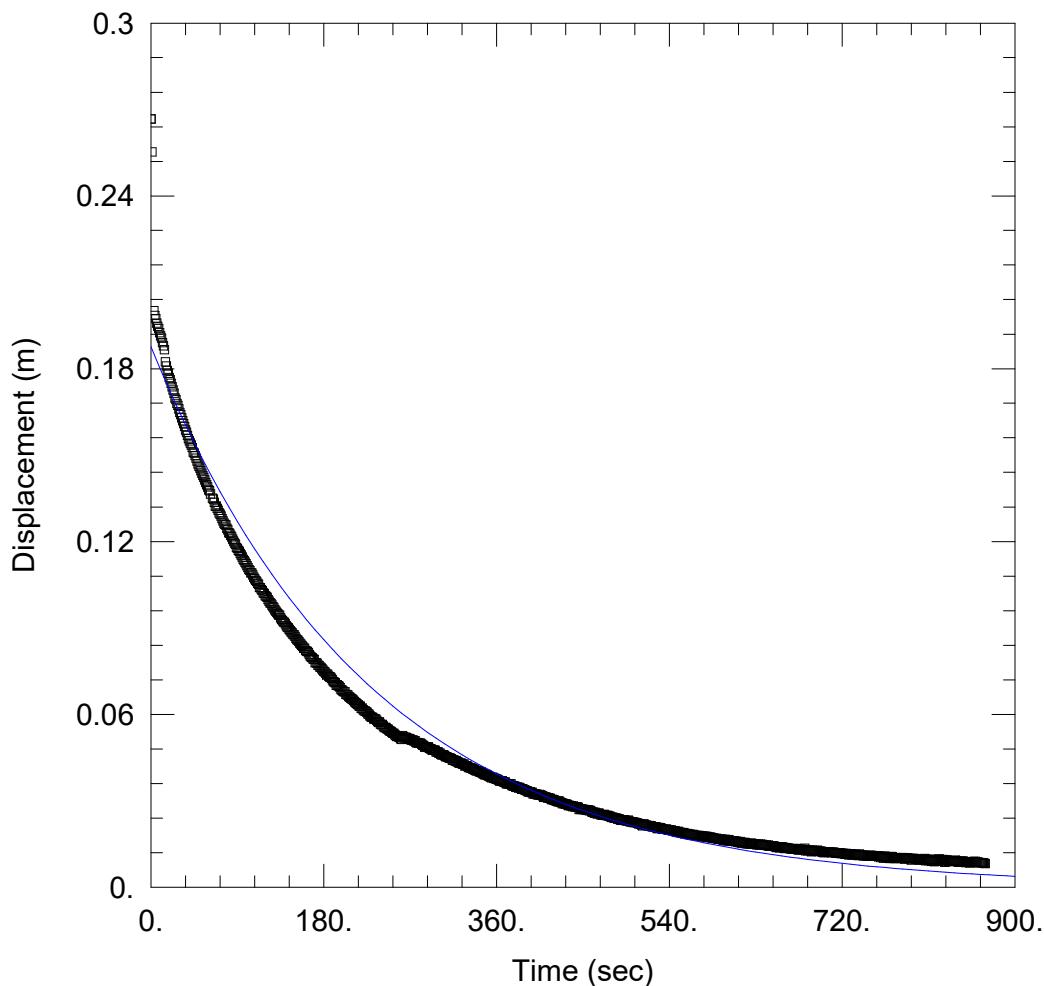
SOLUTION

Aquifer Model: Unconfined

Solution Method: Hvorslev

$K = 1.353E-5$ m/sec

$y_0 = 0.1786$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Slug 2 Linear vs draw.aqt
 Date: 03/31/25

Time: 11:11:53

PROJECT INFORMATION

Company: CIMA+
 Project: T001592B
 Location: Blind River, ON
 Test Well: BH-102
 Test Date: 2024-11-21

AQUIFER DATA

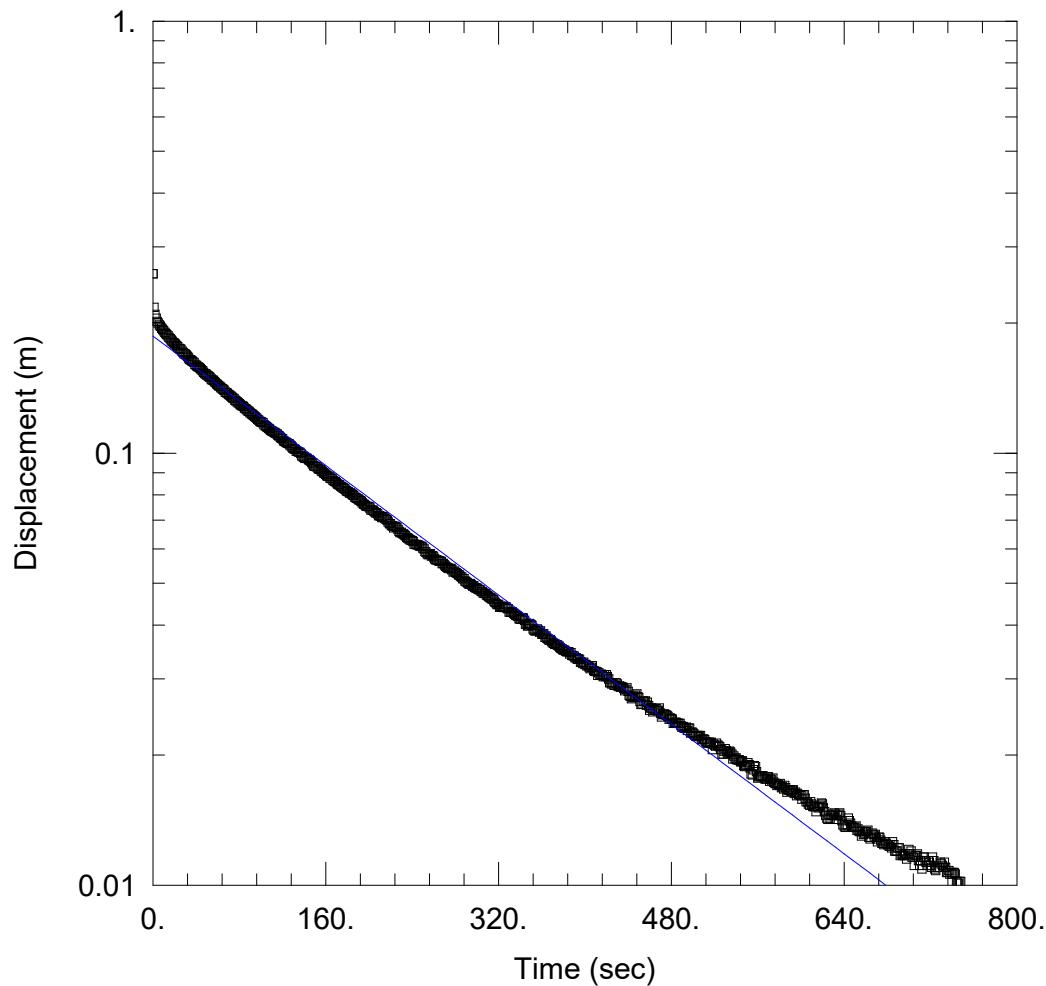
Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: 0.2667 m Static Water Column Height: 3.57 m
 Total Well Penetration Depth: 2.034 m Screen Length: 1.524 m
 Casing Radius: 0.0505 m Well Radius: 0.0254 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Hvorslev
 $K = 1.487E-5$ m/sec $y_0 = 0.1876$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Bail 2 Hvorslev unconfined.aqt
 Date: 03/31/25 Time: 11:19:58

PROJECT INFORMATION

Company: CIMA+
 Project: T001592B
 Location: Blind River, ON
 Test Well: BH-102
 Test Date: 2024-11-21

AQUIFER DATA

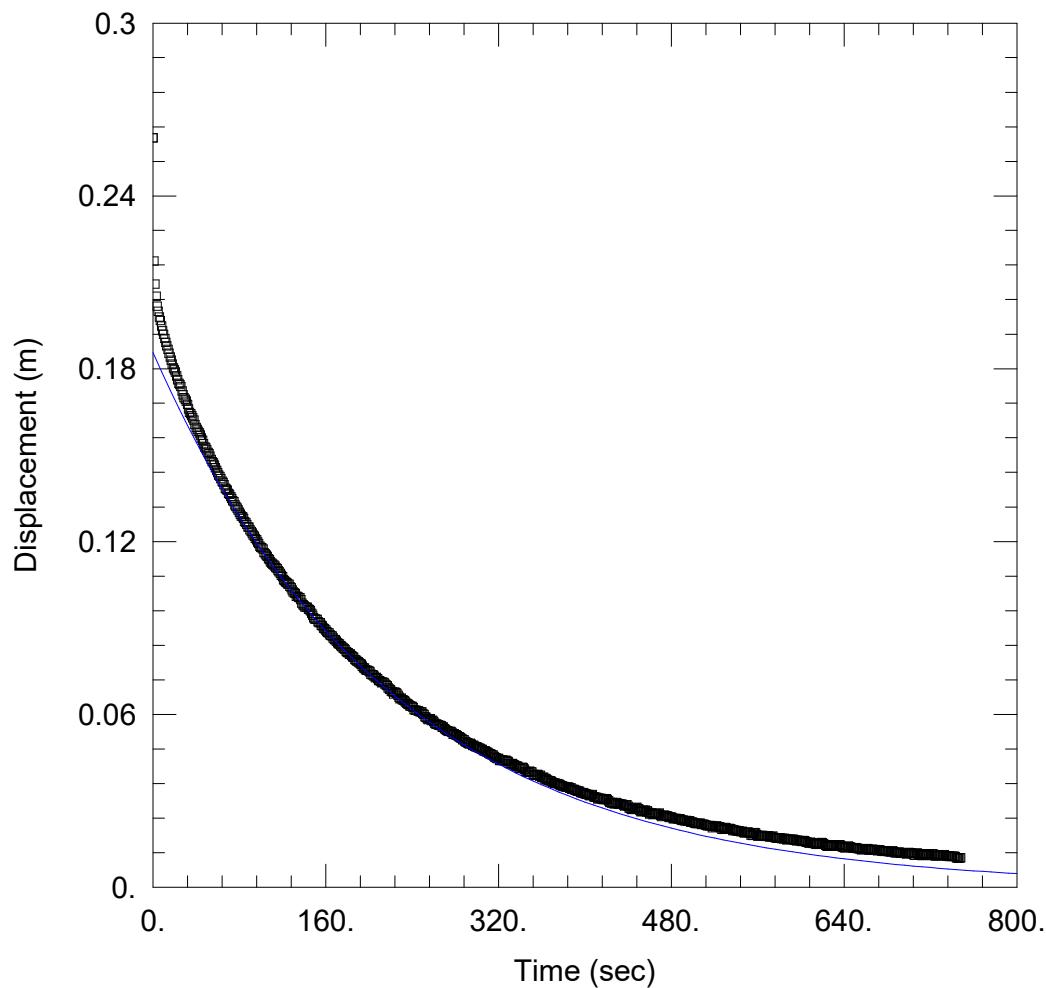
Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: <u>0.2602</u> m	Static Water Column Height: <u>3.57</u> m
Total Well Penetration Depth: <u>2.034</u> m	Screen Length: <u>1.524</u> m
Casing Radius: <u>0.0505</u> m	Well Radius: <u>0.0254</u> m

SOLUTION

Aquifer Model: <u>Unconfined</u>	Solution Method: <u>Hvorslev</u>
$K = 1.477E-5$ m/sec	$y_0 = 0.1866$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Bail 2 Linear vs draw.aqt
 Date: 03/31/25

Time: 11:17:12

PROJECT INFORMATION

Company: CIMA+
 Project: T001592B
 Location: Blind River, ON
 Test Well: BH-102
 Test Date: 2024-11-21

AQUIFER DATA

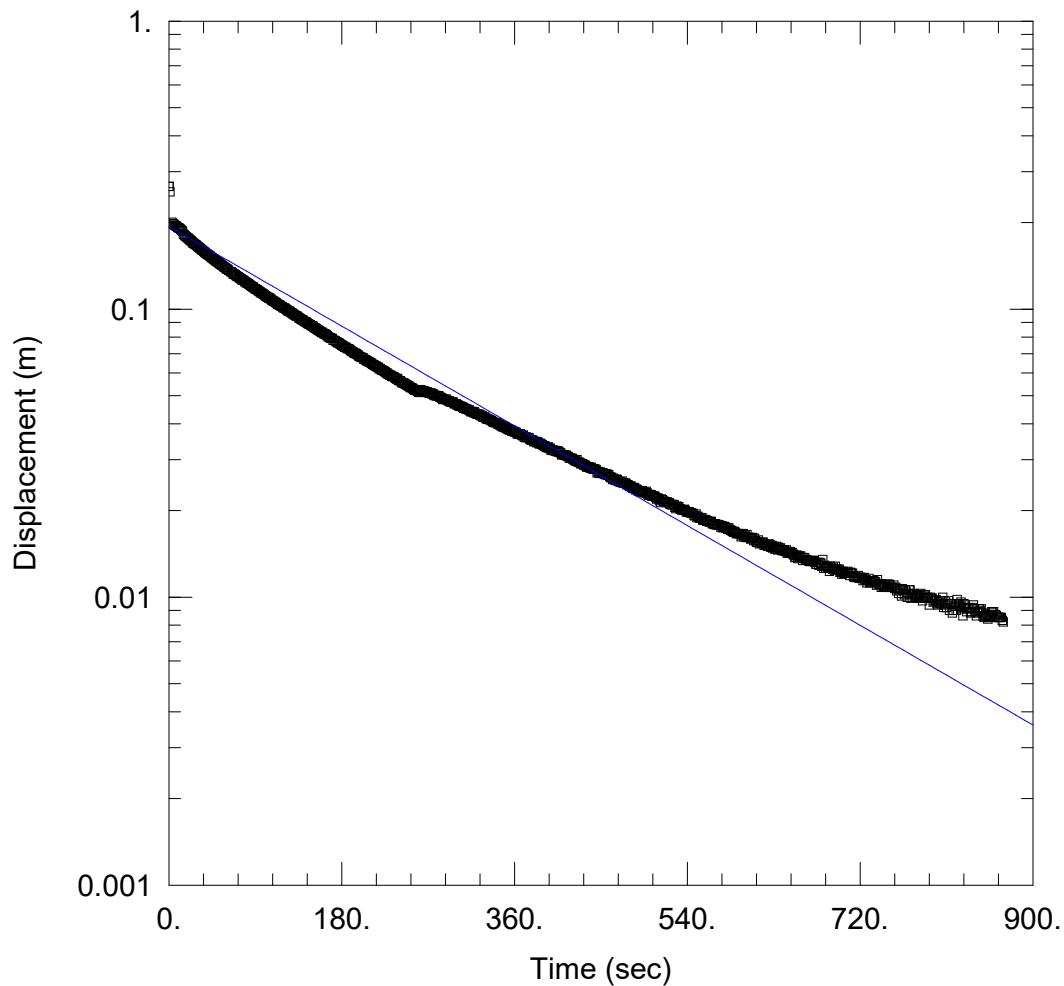
Saturated Thickness: 3.57 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: <u>0.2602</u> m	Static Water Column Height: <u>3.57</u> m
Total Well Penetration Depth: <u>2.034</u> m	Screen Length: <u>1.524</u> m
Casing Radius: <u>0.0505</u> m	Well Radius: <u>0.0254</u> m

SOLUTION

Aquifer Model: <u>Unconfined</u>	Solution Method: <u>Hvorslev</u>
$K = 1.574E-5$ m/sec	$y_0 = 0.1856$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-102 Slug 2 Hvorslev unconfined.aqt

Date: 03/31/25

Time: 11:13:43

PROJECT INFORMATION

Company: CIMA+

Project: T001592B

Location: Blind River, ON

Test Well: BH-102

Test Date: 2024-11-21

AQUIFER DATA

Saturated Thickness: 3.57 m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-102)

Initial Displacement: 0.2667 m

Static Water Column Height: 3.57 m

Total Well Penetration Depth: 2.034 m

Screen Length: 1.524 m

Casing Radius: 0.0505 m

Well Radius: 0.0254 m

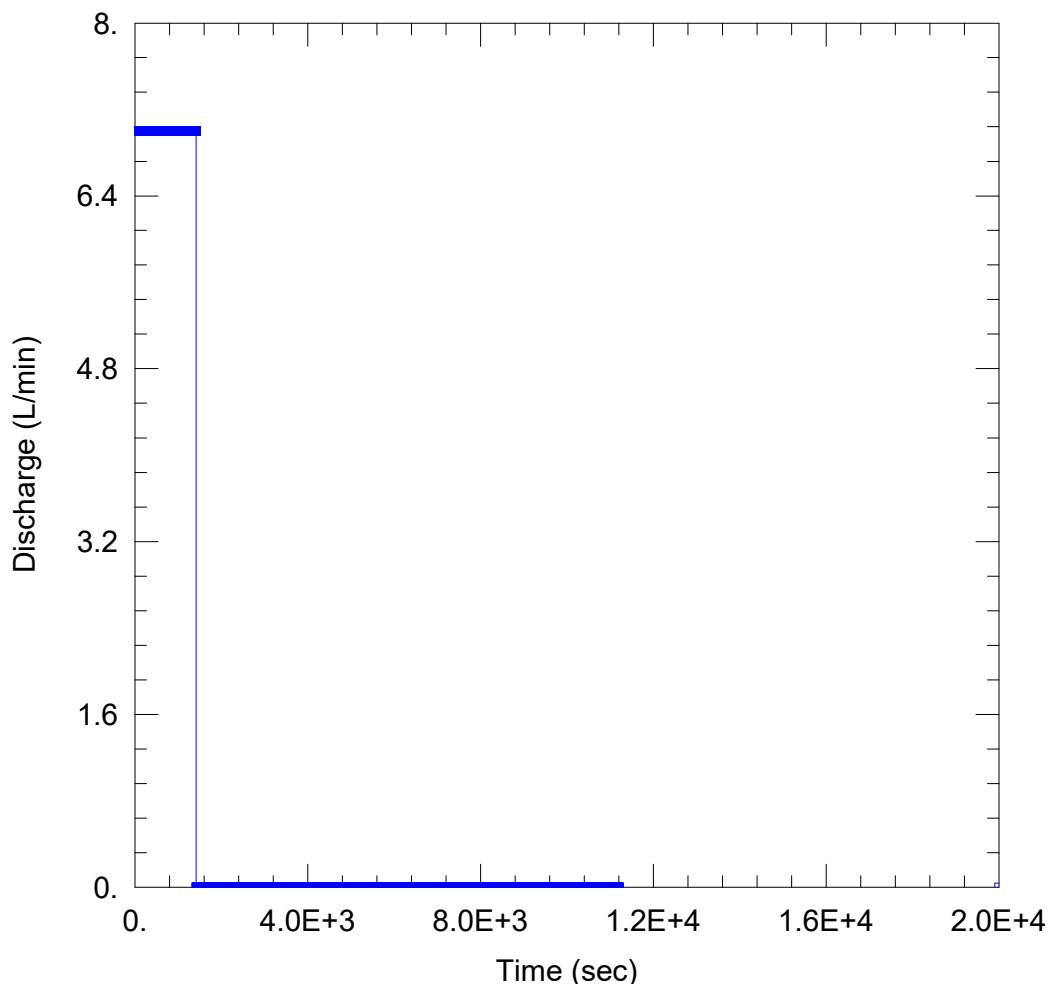
SOLUTION

Aquifer Model: Unconfined

Solution Method: Hvorslev

$K = 1.517E-5$ m/sec

$y_0 = 0.1937$ m



WELL TEST ANALYSIS

Data Set: C:\...\BH-100 Pumping Test - Theis Recovery.aqt
 Date: 04/02/25 Time: 14:23:13

PROJECT INFORMATION

Company: CIMA+
 Project: T00159B
 Location: Blind River
 Test Well: BH-100
 Test Date: 2024-11-21

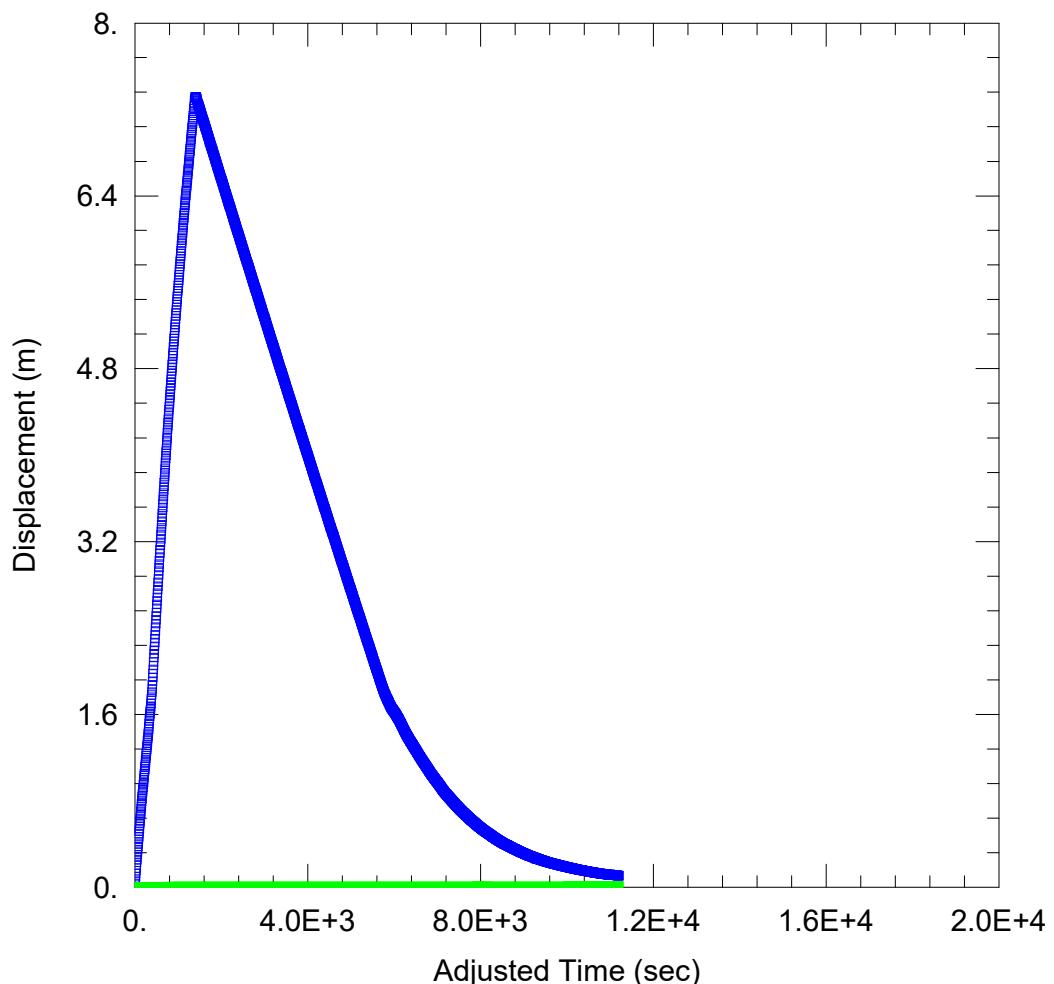
AQUIFER DATA

Saturated Thickness: 7.99 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells		
Well Name	X (m)	Y (m)
<input checked="" type="checkbox"/> BH-100	349246	5116289

Observation Wells		
Well Name	X (m)	Y (m)
BH-100	349246	5116289
BH-101	349282	5116294



WELL TEST ANALYSIS

Data Set: C:\...\BH-100 Pumping Test - Theis Recovery.aqt
 Date: 04/02/25 Time: 14:24:29

PROJECT INFORMATION

Company: CIMA+
 Project: T00159B
 Location: Blind River
 Test Well: BH-100
 Test Date: 2024-11-21

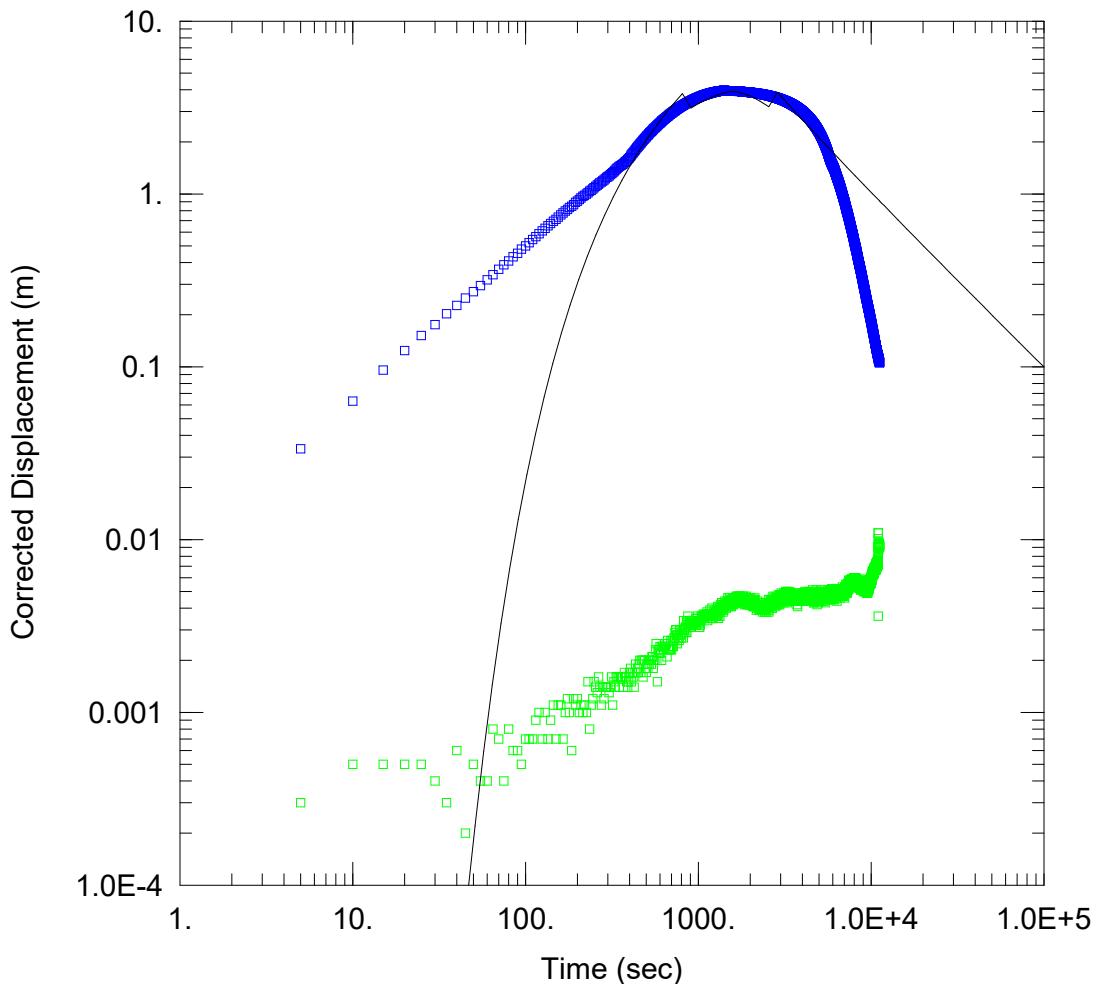
AQUIFER DATA

Saturated Thickness: 7.99 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells		
Well Name	X (m)	Y (m)
BH-100	349246	5116289

Observation Wells		
Well Name	X (m)	Y (m)
□ BH-100	349246	5116289
□ BH-101	349282	5116294



WELL TEST ANALYSIS

Data Set: C:\...\BH-100 Pumping Test 2 - with Pumping and Recovery.aqt
 Date: 04/15/25 Time: 13:57:12

PROJECT INFORMATION

Company: CIMA+
 Project: T00159B
 Location: Blind River
 Test Well: BH-100
 Test Date: 2024-11-21

WELL DATA

Pumping Wells

Well Name	X (m)	Y (m)
BH-100	349246	5116289

Observation Wells

Well Name	X (m)	Y (m)
BH-100	349246	5116289
BH-101	349282	5116294

SOLUTION

Aquifer Model: Unconfined

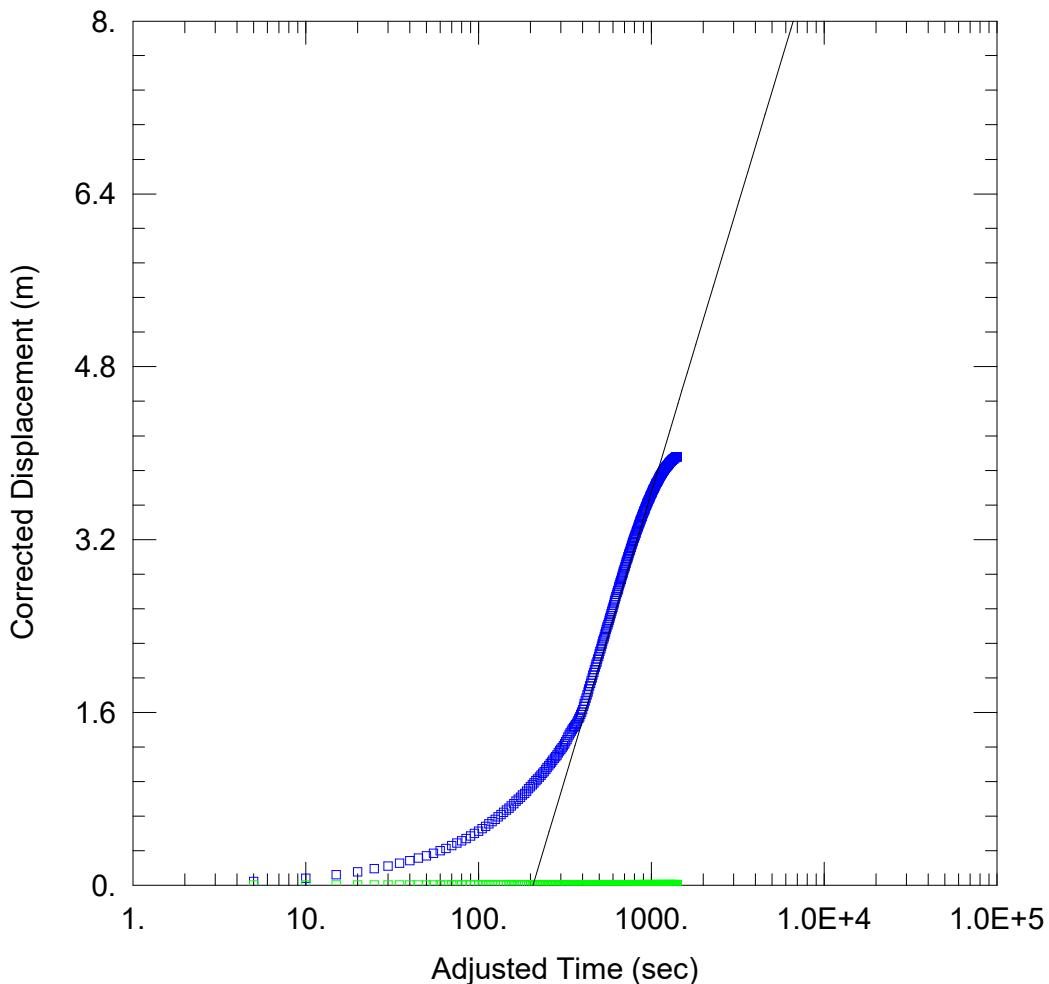
$T = 1.322E-6 \text{ m}^2/\text{sec}$

$Kz/Kr = 1583.2$

Solution Method: Theis

$S = 0.5468$

$b = 7.99 \text{ m}$



WELL TEST ANALYSIS

Data Set: C:\...\BH-100 Pumping Test 2 - with Pumping and Recovery.aqt
 Date: 04/15/25 Time: 13:59:29

PROJECT INFORMATION

Company: CIMA+
 Project: T00159B
 Location: Blind River
 Test Well: BH-100
 Test Date: 2024-11-21

AQUIFER DATA

Saturated Thickness: 7.99 m Anisotropy Ratio (Kz/Kr): 1583.2

WELL DATA

Pumping Wells		
Well Name	X (m)	Y (m)
BH-100	349246	5116289

Observation Wells		
Well Name	X (m)	Y (m)
□ BH-100	349246	5116289
□ BH-101	349282	5116294

SOLUTION

Aquifer Model: Unconfined

$T = 4.027E-6 \text{ m}^2/\text{sec}$

Solution Method: Cooper-Jacob

$S = 0.4633$

D

Appendix D Tables

Appendix D, Table 1 - Soil Quality Analytical Results

T001592B - Hydrogeological Investigation - New Water Intake & Huron Street Reconstruction, Blind River, ON



Parameter	Units	RDL	PWQO and IPWQOs ¹	Sample ID	BH-100	BH-102
					Date	2024-11-21
Parameters						
Total Antimony	mg/L	0.003	0.02		<0.003	<0.003
Total Arsenic	mg/L	0.003c - 0.006d	0.1		<0.003	0.013
Total Barium	mg/L	0.002c - 0.004d	-		0.016	0.197
Total Beryllium	mg/L	0.001c - 0.002d	0.011 ^a		<0.001	<0.002
Total Boron	mg/L	0.01c - 0.02d	0.2 ^b		0.052	0.08
Total Cadmium	mg/L	0.0001c - 0.0002d	0.0002		<0.0001	<0.0002
Total Chromium	mg/L	0.003c - 0.006d	-		0.011	0.019
Total Cobalt	mg/L	0.0005c - 0.0010d	0.0009 ^b		0.0051	0.04
Total Copper	mg/L	0.002c - 0.004d	0.005		0.007	0.033
Total Lead	mg/L	0.0005c - 0.0010d	0.025 ^b		<0.0005	0.0085
Total Molybdenum	mg/L	0.002c - 0.004d	0.04 ^b		0.017	<0.004
Total Nickel	mg/L	0.003c - 0.006d	0.025		0.016	0.019
Total Selenium	mg/L	0.002c - 0.004d	0.1		<0.002	<0.004
Total Silver	mg/L	0.0001c - 0.0002d	0.0001		0.0002	<0.0002
Total Thallium	mg/L	0.0003c - 0.0006d	0.0003 ^b		<0.0003	<0.0006
Total Uranium	mg/L	0.0005c - 0.0010d	0.005 ^b		<0.0005	0.0014
Total Vanadium	mg/L	0.002c - 0.004d	0.006 ^b		<0.002	0.037
Total Zinc	mg/L	0.02c - 0.04d	0.03		<0.020	<0.040
Chromium VI	mg/L	0.001	0.001		<0.001	0.003
Total Mercury	mg/L	0.0001	0.0002		<0.0001	<0.0001
Chloride	mg/L	0.10c - 0.12d	-		78	160
Total Phosphorus	mg/L	0.02c - 0.06d	0.02 ^b		<0.02	0.61
Phenols	mg/L	0.001	0.001		0.001	0.001
Cyanide, WAD	mg/L	0.002c - 0.005d	0.005		<0.002	<0.002
Hardness (as CaCO ₃) (Calculated)	mg/L	0.5	-		33.7	85.7
Total Sodium	mg/L	0.10	-		49.5	78.8
Electrical Conductivity	uS/cm	2	-		388	838
pH	ph Units	NA	6.5 to 8.5		7.16	6.87
Total Suspended Solids	mg/L	10	-		51	3800
Alkalinity (as CaCO ₃)	mg/L	5	-		98	193

Notes:

1 - Reproduced from the Ontario Ministry of Environment and Energy (MOEE, 1994)

Water Management Policies, Guidelines, Provincial Water Quality Objectives

a - Value calculated using equation outlined in the PWQO and IPWQO Guidelines

b - Interim PWQO

c - Reported Detection Limit for samples collected at BH-100

d - Reported Detection Limit for samples collected at BH-102

RDL - Reported Detection Limit

PWQO - Provincial Water Quality Objectives

IPWQO - Interim Provincial Water Quality Objectives

123 - Exceeds Provincial Water Quality or Interim Provincial Water Quality Objectives

E

Appendix E Laboratory Certificate of Analysis

CLIENT NAME: CIMA CANADA INC.
600-1400 BLAIR TOWERS PLACE
OTTAWA, ON K1J 9B8
(613) 860-1870

ATTENTION TO: Mathieu Klein

PROJECT: T001592B

AGAT WORK ORDER: 24Z224690

WATER ANALYSIS REVIEWED BY: Nivine Basily, Inorganic Team Lead

DATE REPORTED: Dec 03, 2024

PAGES (INCLUDING COVER): 7

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*Notes

Disclaimer:

- All work conducted herein has been done using accepted standard protocols, and generally accepted practices and methods. AGAT test methods may incorporate modifications from the specified reference methods to improve performance.
- All samples will be disposed of within 30 days after receipt unless a Long Term Storage Agreement is signed and returned. Some specialty analysis may be exempt, please contact your Client Project Manager for details.
- AGAT's liability in connection with any delay, performance or non-performance of these services is only to the Client and does not extend to any other third party. Unless expressly agreed otherwise in writing, AGAT's liability is limited to the actual cost of the specific analysis or analyses included in the services.
- This Certificate shall not be reproduced except in full, without the written approval of the laboratory.
- The test results reported herewith relate only to the samples as received by the laboratory.
- Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, warranties of merchantability, fitness for a particular purpose, or non-infringement. AGAT assumes no responsibility for any errors or omissions in the guidelines contained in this document.
- All reportable information is available on request from AGAT Laboratories, in accordance with ISO/IEC 17025:2017, ISO/IEC 17025:2005 (Quebec), DR-12-PALA and/or NELAP Standards.
- This document is signed by an authorized signatory who meets the requirements of the MELCCFP, CALA, CCN and NELAP.
- For environmental samples in the Province of Quebec: The analysis is performed on and results apply to samples as received. A temperature above 6°C upon receipt, as indicated in the Sample Reception Notification (SRN), could indicate the integrity of the samples has been compromised if the delay between sampling and submission to the laboratory could not be minimized.



CLIENT NAME: CIMA CANADA INC.

SAMPLING SITE:

Certificate of Analysis

AGAT WORK ORDER: 24Z224690

PROJECT: T001592B

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

ATTENTION TO: Mathieu Klein

SAMPLED BY: Jeremy T.

Inorganic Chemistry (Water)

DATE RECEIVED: 2024-11-22

DATE REPORTED: 2024-12-03

Parameter	Unit	SAMPLE DESCRIPTION:		BH24-100	BH24-102	RDL
		SAMPLE TYPE:	DATE SAMPLED:	Water	Water	
				2024-11-21	2024-11-21	
Total Antimony	mg/L	0.020	0.003	<0.003	0.003	<0.003
Total Arsenic	mg/L	0.1	0.003	<0.003	0.006	0.013
Total Barium	mg/L	0.002		0.016	0.004	0.197
Total Beryllium	mg/L	*	0.001	<0.001	0.002	<0.002
Total Boron	mg/L	0.2	0.010	0.052	0.020	0.080
Total Cadmium	mg/L	0.0002	0.0001	<0.0001	0.0002	<0.0002
Total Chromium	mg/L	0.003		0.011	0.006	0.019
Total Cobalt	mg/L	0.0009	0.0005	0.0051	0.0010	0.0400
Total Copper	mg/L	0.005	0.002	0.007	0.004	0.033
Total Lead	mg/L	*	0.0005	<0.0005	0.0010	0.0085
Total Molybdenum	mg/L	0.040	0.002	0.017	0.004	<0.004
Total Nickel	mg/L	0.025	0.003	0.016	0.006	0.019
Total Selenium	mg/L	0.1	0.002	<0.002	0.004	<0.004
Total Silver	mg/L	0.0001	0.0001	0.0002	0.0002	<0.0002
Total Thallium	mg/L	0.0003	0.0003	<0.0003	0.0006	<0.0006
Total Uranium	mg/L	0.005	0.0005	<0.0005	0.0010	0.0014
Total Vanadium	mg/L	0.006	0.002	<0.002	0.004	0.037
Total Zinc	mg/L	0.030	0.020	<0.020	0.040	<0.040
Chromium VI	mg/L	0.001	0.001	<0.001	0.001	0.003
Total Mercury	mg/L		0.0001	<0.0001	0.0001	<0.0001
Chloride	mg/L		0.10	78.0	0.12	160
Total Phosphorus	mg/L	*	0.02	<0.02	0.06	0.61
Phenols	mg/L	0.001	0.001	0.001	0.001	0.001
Cyanide, WAD	mg/L	0.005	0.002	<0.002	0.002	<0.002
Hardness (as CaCO ₃) (Calculated)	mg/L	0.5	33.7	0.5	85.7	
Total Sodium	mg/L	0.10	49.5	0.20	78.8	
Electrical Conductivity	uS/cm	2	388	2	838	
pH	pH Units	6.5-8.5	NA	7.16	NA	6.87
Total Suspended Solids	mg/L	10	51	10	3800	
Alkalinity (as CaCO ₃)	mg/L	5	98	5	193	

Certified By:



Nivine Basily



CLIENT NAME: CIMA CANADA INC.

SAMPLING SITE:

Laboratories

Certificate of Analysis

AGAT WORK ORDER: 24Z224690

PROJECT: T001592B

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

ATTENTION TO: Mathieu Klein

SAMPLED BY: Jeremy T.

Inorganic Chemistry (Water)

DATE RECEIVED: 2024-11-22

DATE REPORTED: 2024-12-03

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to PWQO ^ Variable - refer to guideline reference document
Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.
6357647 Dilution required, RDL has been increased accordingly.
Analysis performed at AGAT Toronto (unless marked by *)



Mathieu Klein



Exceedance Summary

AGAT WORK ORDER: 24Z224690

PROJECT: T001592B

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

CLIENT NAME: CIMA CANADA INC.

ATTENTION TO: Mathieu Klein

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
6357630	BH24-100	ON PWQO	Inorganic Chemistry (Water)	Total Cobalt	mg/L	0.0009	0.0051
6357630	BH24-100	ON PWQO	Inorganic Chemistry (Water)	Total Copper	mg/L	0.005	0.007
6357630	BH24-100	ON PWQO	Inorganic Chemistry (Water)	Total Silver	mg/L	0.0001	0.0002
6357647	BH24-102	ON PWQO	Inorganic Chemistry (Water)	Chromium VI	mg/L	0.001	0.003
6357647	BH24-102	ON PWQO	Inorganic Chemistry (Water)	Total Cobalt	mg/L	0.0009	0.0400
6357647	BH24-102	ON PWQO	Inorganic Chemistry (Water)	Total Copper	mg/L	0.005	0.033
6357647	BH24-102	ON PWQO	Inorganic Chemistry (Water)	Total Vanadium	mg/L	0.006	0.037



Quality Assurance

CLIENT NAME: CIMA CANADA INC.

PROJECT: T001592B

SAMPLING SITE:

AGAT WORK ORDER: 24Z224690

ATTENTION TO: Mathieu Klein

SAMPLED BY: Jeremy T.

Water Analysis

RPT Date: Dec 03, 2024			DUPLICATE			Method Blank	REFERENCE MATERIAL		METHOD BLANK SPIKE			MATRIX SPIKE				
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits		
								Lower	Upper			Lower		Recovery	Lower	Upper

Inorganic Chemistry (Water)

Total Antimony	6354888	<0.003	<0.003	NA	< 0.003	104%	70%	130%	101%	80%	120%	102%	70%	130%	
Total Arsenic	6354888	<0.003	<0.003	NA	< 0.003	100%	70%	130%	98%	80%	120%	103%	70%	130%	
Total Barium	6354888	<0.002	<0.002	NA	< 0.002	105%	70%	130%	103%	80%	120%	107%	70%	130%	
Total Beryllium	6354888	<0.001	<0.001	NA	< 0.001	105%	70%	130%	96%	80%	120%	94%	70%	130%	
Total Boron	6354888	<0.010	<0.010	NA	< 0.010	103%	70%	130%	100%	80%	120%	99%	70%	130%	
Total Cadmium	6354888	<0.0001	<0.0001	NA	< 0.0001	99%	70%	130%	97%	80%	120%	99%	70%	130%	
Total Chromium	6354888	<0.003	<0.003	NA	< 0.003	99%	70%	130%	103%	80%	120%	104%	70%	130%	
Total Cobalt	6354888	<0.0005	<0.0005	NA	< 0.0005	104%	70%	130%	98%	80%	120%	102%	70%	130%	
Total Copper	6354888	<0.002	<0.002	NA	< 0.002	99%	70%	130%	99%	80%	120%	102%	70%	130%	
Total Lead	6354888	<0.0005	<0.0005	NA	< 0.0005	96%	70%	130%	93%	80%	120%	98%	70%	130%	
Total Molybdenum	6354888	<0.002	<0.002	NA	< 0.002	102%	70%	130%	82%	80%	120%	102%	70%	130%	
Total Nickel	6354888	<0.003	<0.003	NA	< 0.003	94%	70%	130%	101%	80%	120%	100%	70%	130%	
Total Selenium	6354888	<0.002	<0.002	NA	< 0.002	103%	70%	130%	103%	80%	120%	106%	70%	130%	
Total Silver	6354888	<0.0001	<0.0001	NA	< 0.0001	100%	70%	130%	94%	80%	120%	104%	70%	130%	
Total Thallium	6354888	<0.0003	<0.0003	NA	< 0.0003	108%	70%	130%	99%	80%	120%	104%	70%	130%	
Total Uranium	6354888	<0.0005	<0.0005	NA	< 0.0005	102%	70%	130%	99%	80%	120%	108%	70%	130%	
Total Vanadium	6354888	<0.002	<0.002	NA	< 0.002	99%	70%	130%	106%	80%	120%	107%	70%	130%	
Total Zinc	6354888	<0.020	<0.020	NA	< 0.020	101%	70%	130%	99%	80%	120%	99%	70%	130%	
Chromium VI	6358830	0.003	0.005	NA	< 0.001	98%	70%	130%	88%	80%	120%	92%	70%	130%	
Total Mercury	6354888	<0.0001	<0.0001	NA	< 0.0001	100%	70%	130%	102%	80%	120%	101%	70%	130%	
Chloride	6364160	365	360	1.4%	< 0.10	96%	70%	130%	101%	80%	120%	NA	70%	130%	
Total Phosphorus	6356285	0.11	0.10	NA	0.03	100%	70%	130%	97%	80%	120%	NA	70%	130%	
Phenols	6362562	<0.001	<0.001	NA	< 0.001	94%	90%	110%	94%	90%	110%	100%	80%	120%	
Cyanide, WAD	6357630	6357630	<0.002	<0.002	NA	< 0.002	90%	70%	130%	94%	80%	120%	104%	70%	130%
Total Sodium	6354888	33.7	33.6	0.3%	< 0.10	105%	70%	130%	103%	80%	120%	121%	70%	130%	
Electrical Conductivity	6361855	1600	1600	0.0%	< 2	98%	80%	120%							
pH	6361855	7.69	7.72	0.4%	NA	100%	90%	110%							
Total Suspended Solids	6362562	<10	<10	NA	< 10	102%	80%	120%							
Alkalinity (as CaCO ₃)	6361855	242	248	2.4%	< 5	100%	80%	120%							

Comments: NA signifies Not Applicable.

Duplicate NA: results are under 5X the RDL and will not be calculated.

Matrix spike NA: Spike level < native concentration. Matrix spike acceptance limits do not apply and are not calculated.



Nivine Basil

Certified By:



Method Summary

CLIENT NAME: CIMA CANADA INC.

PROJECT: T001592B

SAMPLING SITE:

AGAT WORK ORDER: 24Z224690

ATTENTION TO: Mathieu Klein

SAMPLED BY: Jeremy T.

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Water Analysis			
Total Antimony	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Arsenic	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Barium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Beryllium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Boron	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cadmium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Chromium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cobalt	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Copper	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Lead	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Molybdenum	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Nickel	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Selenium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Silver	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Thallium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Uranium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Vanadium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Zinc	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Chromium VI	INOR-93-6073	modified from SM 3500-CR B	SPECTROPHOTOMETER
Total Mercury	MET-93-6100	modified from EPA 245.2 and SM 3112 B	CVAAS
Chloride	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Total Phosphorus	INOR-93-6022	modified from SM 4500-P B and SM 4500-P E	SPECTROPHOTOMETER
Phenols	INOR-93-6072	mod from SM 510C, EPA 420.2, ISO 3696, ASTM D1193	SEGMENTED FLOW ANALYSIS
Cyanide, WAD	INOR-93-6052	modified from ON MOECC E3015, SM 4500-CN- I, G-387	SEGMENTED FLOW ANALYSIS
Hardness (as CaCO ₃) (Calculated)	MET-93-6105	modified from EPA SW-846 6010C & 200.7 & SM 2340 B	CALCULATION
Total Sodium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP/MS
Electrical Conductivity	INOR-93-6000	SM 2510 B	PC TITRATE
pH	INOR-93-6000	modified from SM 4500-H+ B	PC TITRATE
Total Suspended Solids	INOR-93-6028	modified from EPA 1684, ON MOECC E3139, SM 2540B,C,D	BALANCE
Alkalinity (as CaCO ₃)	INOR-93-6000	Modified from SM 2320 B	PC TITRATE

F

Appendix F Dewatering Calculations

Appendix F.1: Infiltration Rates - Blind River LLPS

Overburden Unit		Bedrock Unit	
Datum		Datum	
Ground Surface Elevation (m asl)	180.56 m asl	Bedrock Surface Elevation (m asl)	175.01 m asl
Bottom excavation elevation (m asl)	175.01 m asl	Bottom excavation elevation (m asl)	173.39 m asl
Groundwater elevation (m)	179.56 masl	Groundwater elevation (m)	175.01 masl
Required drawdown from bottom of excavation (m)	0.0 m	Drawdown from bottom of excavation (m)	1 m
Elevation at required drawdown (m asl)	175.01 m asl	Elevation at required drawdown (m asl)	172.39 m asl
INPUTS		INPUTS	
<u>Dimensions</u>		<u>Dimensions</u>	
Length excavation (m):	13.5 m	Length excavation (m):	13.5 m
Width excavation (m):	13 m	Width excavation (m):	13 m
Area of excavation :	176 m ²	Area of excavation :	176 m ²
<u>Hydraulic Conductivity</u>		<u>Hydraulic Conductivity</u>	
hydraulic conductivity:	1.54E-05 m/s	hydraulic conductivity:	2.67E-07 m/s
Drawdown total drawdown (m)	4.55 m	Drawdown total drawdown (m)	2.62 m
<u>Radius of influence</u>		<u>Radius of influence</u>	
Radius of influence (Ro1) :	53.6 m	Radius of influence (Ro1) :	4.1 m
<u>Effective Radius</u>		<u>Effective Radius</u>	
Effective radius of point source (rw1) :	7.5 m	Effective radius of point source (rw1) :	7.5 m
RESULTS			
<u>Infiltration rates</u>		<u>Infiltration rates</u>	
Infiltration rate in excavation:	4.77E-04 m ³ /s	Infiltration rate in excavation:	1.33E-05 m ³ /s
Infiltration rate in excavation :	41.21 m ³ /day	Infiltration rate in excavation :	1.15 m ³ /day
	28.6 L/min		0.8 L/min
<u>Safety factor of 3 on K</u>		<u>Safety factor of 3 on K</u>	
Safety factor on K :	3	Safety factor on K :	3
Infiltration rate in excavation (Q) :	1.43E-03 m ³ /s	Infiltration rate in excavation (Q) :	3.98E-05 m ³ /s
Infiltration rate in excavation (Q) :	123.62 m ³ /day	Infiltration rate in excavation (Q) :	3.44 m ³ /day
		<u>Total Excavation Infiltration Rate</u>	42.35 m ³ /day 29.4 L/min
		<u>Safety factor of 3 on K</u>	127.06 m ³ /day 88.2 L/min